

1 **URBAN WET-WEATHER FLOWS**

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3  
4 **ABSTRACT/INTRODUCTION**

5 This subject, Urban Wet-Weather Flows, was comprised of three basic subareas – combined-sewer  
6 overflows (CSOs), sanitary-sewer overflows (SSOs), and stormwater discharges. Major proceedings  
7 related to wet-weather flow (WWF) published during 2000 were the following: (1) ASCE EWRI  
8 Conference – Bridging the Gap: Meeting the World’s Water and Environmental Resources Challenges  
9 (ASCE, 2001); (2) WEFTEC 2001, 74th Annual Conference and Exposition (WEF, 2001); (3) 5<sup>th</sup>  
10 International Conference: Diffuse/Nonpoint Pollution and Watershed Management (IWA, 2001); (4)  
11 Models and Applications to Urban Water Systems, Monograph 9 (CHI, 2001); (5) 2001 A Collection  
12 Systems Odyssey: Integrating O&M and Wet Weather Solutions (WEF, 2001); (6) 7<sup>th</sup> Annual Industrial  
13 Wastes Technical and Regulatory Conference (WEF, 2001); and (7) Proceedings of the Third  
14 International Conference on Watershed Management (ASCE, 2001).

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16 **STORMWATER CHARACTERIZATION**

17 *General*

18 Fan et al. (2001b) reviewed the transport of toxic pollutants through multiple media and drainage systems  
19 in the urban watershed during wet-weather periods. Field studies have identified that a major portion of  
20 hazardous waste priority pollutants including benzene, polynuclear aromatic hydrocarbons (PAHs),  
21 polychlorinated biphenyls (PCBs), pesticides, and heavy metals (e.g., arsenic, cadmium, chromium,  
22 copper, lead, mercury, and zinc) contained in urban stormwater runoff are in particulate form or sorbed  
23 onto particles. Fatoki and Mathabatha (2001) investigated the distribution of heavy metals (zinc,  
24 cadmium, copper, iron, manganese and lead) in seawater and in sediment samples from the East London  
25 and Port Elizabeth harbors. The results indicate the contribution of heavy metal pollution from storm  
26 drains. Ship repair activities were also suspected to a source of elevated concentrations in the upper  
27 reaches of the harbor. Kayhanian et al. (2001b) analyzed the impact of ‘non-detects’ or ‘NDs’ on  
28 stormwater data because dissolved fractions of stormwater constituents often are not detected above  
29 laboratory reporting limits. Analysts and stormwater modelers have represented these NDs in stormwater  
30 data sets using a variety of methods. In the paper, different methods of data analysis were used to  
31 determine constituent mass concentrations from water quality datasets that include ND values.  
32 Depending on the number of NDs and the method of data analysis, differences ranging from 1 to 70  
33 percent were in mean values, which would have significant impacts on estimations of constituent mass  
34 loading.

35 *Rainfall Monitoring and Urban Hydrology*

36 Current forecasting systems from meteorological offices have not been well suited for accurate rainfall  
37 forecast in urban areas (Aspergren et al., 2001). This project provided a short-term small-scale prediction  
38 of rain based on radar images. The extrapolation part of the methodology, based on a sophisticated cross  
39 correlation of images, was optimized by a neural network technique. Three different application sites in  
40 Europe have been used to validate the system. Burian et al. (2001a) reported on one technique for  
41 disaggregating long-term hourly rainfall records into subhourly increments that involved the use of artificial  
42 neural networks (ANNs). The research evaluated the influence on performance of several ANN model  
43 characteristics and training issues including data standardization, geographic location of training data,  
44 quantity of training data, number of training iterations, and the number of hidden neurons in the ANN.  
45 Results suggested that data from rainfall-gauging stations within several hundred kilometers of the station  
46 to be disaggregated would be adequate for training the ANN rainfall disaggregation model.

47 The L-THIA (Long-Term Hydrologic Impact Assessment) model can be used to assess how land-use  
48 changes affect annual average runoff (Bhaduri et al., 2001). Looking at runoff calculations, SWMM was  
49 compared with L-THIA. Applications of L-THIA and SWMM to two small watersheds in Chicago showed  
50 that L-THIA predicts annual average runoff between 1.1 and 23.7% higher than SWMM, and was easier

1 and quicker to use than SWMM. Results suggested that L-THIA could be an appropriate tool for initially  
2 assessing the relative impacts of land-use change scenarios. The runoff coefficient (ratio of total  
3 streamflow volume to the total precipitation over a certain area and time) has been shown to play a  
4 fundamental role in the planning, design and operation of water resources in a catchment (Kadioglu and  
5 Sen, 2001). In this paper, monthly runoff coefficient changes within an annual period were represented  
6 through a simple polygon diagram concept obtained from monthly precipitation and runoff data. The  
7 application of the polygon method was presented for catchments around Istanbul, Turkey. Kojiri et al.  
8 (2001) reviewed the flood management system used for urban rivers in Japan. The system is composed  
9 of three subsystems: an on-line data collection subsystem for collecting rainfall and water level data; a  
10 flood prediction subsystem based on the previous 3-hour hydrologic data; and a results display  
11 subsystem. This system has been used for practical flood prediction. Semadeni-Davies et al. (2001)  
12 investigated the radiation balance of urbanized catchments as it relates to snowmelt. Snowpacks  
13 experience either enhanced or decreased irradiance depending on snowpack location and condition, and  
14 changes to localized irradiance (and melt rates) have implications for urban runoff generation. Net allwave  
15 radiation measurements over snow made in Lulea, Sweden during April 1997 and 1998 were presented.  
16 The results showed that urban structures significantly alter radiation over snow, and therefore, including  
17 snowmelt energetics within design and management techniques is needed.

### 18 *Stormwater Quality*

19 McPherson et al. (2001) compared the relative contributions of dry weather flow (DWF) and wet weather  
20 flow (WWF) from the highly urbanized Ballona Creek watershed (BCW) in Southern California using  
21 empirical and deterministic models. DWF contributed approximately 10 – 30% of the total annual flow  
22 discharged from Ballona Creek, a significant contribution. Yamada et al. (2001c) compared the pollutant  
23 load from runoff that were collected from storm sewer pipes in 7 cities. The contribution of land use, soil,  
24 amount of rainfall, rainfall intensity and antecedent dry period was quantified. Choe et al. (2001) analyzed  
25 surface runoff from selected residential and industrial zones. The event mean concentrations of COD, SS,  
26 TKN, and TP in the residential zone were 313 mg/L, 279 mg/L, 8.45 mg/L, 1.98 mg/L, and those in the  
27 industrial zone were 80 mg/L, 106 mg/L, 5.07 mg/L, and 1.93 mg/L, respectively. The degree of first-  
28 flushing effect was in the following order: TKN > COD > SS > TP > PO<sub>4</sub>-P. The pollutant loads of the  
29 above constituents was highly correlated with SS. The report titled "The removal of urban litter from  
30 stormwater conduits and streams" (by Armitage et al.,) noted that little data was available on the nature  
31 and quantity of litter in stormwater drainage systems (Marais et al., 2001). The Council for Scientific and  
32 Industrial Research estimated in 1991 that 780 000 tonnes of waste a year entered the drainage systems  
33 of South Africa.

34 Fifteen highway construction sites were monitored by the California Department of Transportation  
35 (Caltrans) to assess the runoff quality from the sites (Kayhanian et al., 2001a). The results indicated the  
36 following: (a) construction-site runoff constituent concentrations were less than typical Caltrans and non-  
37 Caltrans highway runoff constituent concentrations, with the exception of total chromium, total nickel, total  
38 phosphorus, total suspended solids (TSS), and turbidity. (b) The concentrations of TSS and turbidity likely  
39 resulted from soil disturbance. (c) The origins of the total chromium, total nickel, and total phosphorus  
40 concentrations are unknown. (d) A correlation was observed between TSS and particulate runoff  
41 concentrations of chromium, copper, and zinc, indicating that solids removal may reduce total metals  
42 concentrations. The Solids Transport and Deposition Study (STDS) characterized the rates and patterns  
43 of solids transfer to, and the collection within, storm water drain inlets located along Caltrans highway  
44 facilities (Quasebarth et al., 2001). The primary objective was to determine if certain distinguishable site  
45 characteristics controlled the transport and deposition of sediment, metals, vegetation, litter, and  
46 petroleum hydrocarbons to highway drain inlets. The ANOVA results indicated that the four primary  
47 factors (erosion control/sediment loading [vegetation factor], litter management [litter factor], toxic  
48 pollutant generation potential [adjacent land use factor], and roadway design [design factor]) likely had  
49 little overall control on solids accumulation or metals mass accumulation, although roadway design and  
50 litter management were possibly important in some cases. Smith (2001) presented a case study of  
51 stormwater and sediment analysis in flood control sumps in an urban watershed. The results suggested  
52 that: (1) first-flush samples may not reflect outfall concentrations of stormwater to the sump; (2) time-  
53 variable concentrations of pollutants in a sump can be related to the hydraulic characteristics of the basin;  
54 and, (3) post-event sediment analysis verified pollutant capture in the sumps. HEC-1 software was used

1 to estimate the flow hydrograph for each outfall to a sump as part of the overall flow balance (Smith et al.,  
2 2001c). The results suggested that HEC-1 calculation provide a satisfactory estimate of the total runoff  
3 and its time-distribution to the sump. The hydraulic model was then used to estimate nonpoint loads of  
4 selected heavy metals to the sump and to the river

5 Atmospheric deposition of particulate matter, organic carbon and PAHs was measured at two stations in  
6 an urban area (Ozaki et al., 2001). The deposition fluxes of particulate matter and organic carbon were  
7 nearly constant over the sampling periods, while PAHs had seasonal variability. The first flush  
8 phenomenon was clearly observed during all the rainy periods. Although the PAHs in runoff agreed with  
9 that of atmospheric deposition, their fluxes were several times larger. Loading of dissolved organic carbon  
10 (DOC) from parking lot storm runoff was found to be a significant, yet relatively neglected, source of  
11 elevated DOC concentration in urban streams (Lee and Schwartz, 2001). This study investigated DOC  
12 transport and loading from an paved parking lot during rainstorm events in order to elucidate impacts of  
13 the stormwater runoff on the chemistry of an urban stream. DOC measurements from the parking lot  
14 showed that the interevent period between storms and the precipitation intensity controlled the maximum  
15 DOC concentration and the time required to reach the maximum DOC concentration, respectively.

16 The fate and transport of metallic pollutants through a watershed were related to the characteristics of the  
17 solid particles to which they are bound (Magnuson et al., 2001). Because the particles most often  
18 associated with metal pollution have nominal diameters of  $< 50 \mu\text{m}$ , split-flow thin-cell (SPLITT)  
19 fractionation was investigated as a means to study the metal loading as a function of particle settling rate.  
20 Sansalone et al. (2001) showed that urban storm water levels of Zn, Cu, Cd, Pb, Cr, and Ni can be  
21 significantly above ambient background levels, and for many urban and transportation land uses, often  
22 exceed surface water discharge criteria for both dissolved and particulate-bound fractions. The authors  
23 advocated a multiple-unit-operation approach to stormwater treatment.

24 Turer et al. (2001) investigated the accumulation of metals in roadside soils at a site for which extensive  
25 runoff data were also available. The results demonstrated that heavy metal contamination in the top 15  
26 cm of the soil was very high compared to local background levels. The maximum measured amount for  
27 Pb was 1980 ppm (at 10-15 cm depth) and for Zn was 1430 ppm (at 0-1 cm depth). The correlation to  
28 organic C is stronger than the correlation to depth. Cluster analysis of the heavy metal data showed that  
29 Pb, Zn and Cu were closely associated to one another, but that Ni and Cr did not show an association  
30 with each other or with either organic C or depth. Mass balance calculations for Pb in soil showed that  
31 most of the Pb came from exhausts of vehicles when leaded gasoline was in use, with about 40% of the  
32 Pb retained in the soil. Zinc and other trace metal (V, Cr, Co, Ni, Cu, Cd, and Pb) concentrations were  
33 measured in the Atlanta metropolitan region and in relatively undeveloped watersheds within the Georgia  
34 Piedmont and Blue Ridge Provinces (Rose et al., 2001). Zinc concentrations in street runoff [median (Zn)  
35 =  $905 \mu\text{g/L}$ ] were significantly greater than zinc concentrations in Peachtree Creek storm runoff [median  
36 (Zn) =  $60 \mu\text{g/L}$ ], which were, in turn, greater than zinc concentrations contaminants within non-storm  
37 baseflow in Peachtree Creek [median (Zn) =  $14 \mu\text{g/L}$ ]. A two end member mass balance model suggested  
38 that a large proportion of the zinc present in the street runoff was adsorbed and transported by the  
39 suspended sediment.

40 Mosley and Peake (2001) characterized urban runoff from a catchment in Dunedin, New Zealand during  
41 base flows and storm flows from five rainfall events. Fe and Pb were found to be predominantly particle-  
42 associated ( $>0.4 \mu\text{m}$ ) with concentrations increasing significantly at the beginning of storm run-off. In  
43 contrast, the majority of Cu and Zn was found in the  $<0.4 \mu\text{m}$  fraction prior to rain but a significant  
44 proportion was present in the  $> 0.4 \mu\text{m}$  fraction during the initial period of storm flows. The results indicate  
45 that Cu and Zn may be more bioavailable, and more difficult to remove by stormwater treatment, than Pb.  
46 The pH level and the concentration of major ions ( $\text{Ca}^{+2}$ ,  $\text{Na}^{+}$ ,  $\text{Mg}^{+2}$ ,  $\text{K}^{+}$ ), dissolved  $\text{PO}_4\text{-P}$ , and  $\text{NO}_3$   
47 generally decreased during storm flows due to rainwater dilution. Concentrations of total N and P often  
48 increased during the initial period of storm run-off, likely because of wash-off of particulate plant material.  
49 Significant amounts of non-point source runoff were shown to enter the Santa Monica Bay from the  
50 Ballona Creek Watershed during wet weather flow. Buffleben et al. (2001) monitored four storms in the  
51 watershed. The watershed is developed mostly with residential, commercial and light industrial land uses.  
52 They found that the suspended solids phase primarily transported the mass for five of the six metals  
53 studied: cadmium, chromium, copper, lead, and nickel. Arsenic was found primarily in the aqueous phase.

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2 **COMBINED SEWER/SANITARY SEWER OVERFLOW CHARACTERIZATION**

3 Hannan et al. (2001) reviewed the ASCE study that developed a guidance manual for identifying Sanitary  
4 Sewer Overflows (SSOs). The project surveyed twelve medium-to-large utilities for their SSO-  
5 identification protocols, with protocols organized into three major categories: Hydraulic, Maintenance and  
6 Inspection, and Structural. King County (Washington) has been assessing inflow and infiltration (I/I) in the  
7 county's service area (Swarner et al., 2001a). The program included rainfall and flow monitoring,  
8 computer modeling, economic analyses, field investigations and pilot projects. The results will be used to  
9 determine the cost effectiveness of removing I/I from the system. This project has developed detailed  
10 information about the location of I/I sources and potential cost-effective improvements to remove or  
11 reduce critical sources of I/I. Inflow/infiltration values were estimated in the South Beaches regional  
12 wastewater collection system (Brevard County, FL) through re-examining pump flow data (Fernandez  
13 2001). Operating station data was used in a custom-designed hydraulic model. Pump operating points  
14 were deduced through an interactive process of balancing flows from tributary pump stations.

15 Ahyerre et al. (2001a) investigated the solids composition of dry weather flow in the "Le Marais" combined  
16 sewer network in Paris after it was noted that there was an increase in the solids concentration in the  
17 lower part of the sewer's flow. The study results showed that the accumulation rate of the organic solids  
18 layer was 215 g/(m<sup>2</sup>-day) with the particles heavily loaded with pollutants (VSS to TSS = 66 – 75%).  
19 Flushing experiments showed that this organic layer is easily eroded by small storm events. A second  
20 publication by Ahyerre et al. (2001b), as part of the same sewer investigation, reported that the erosion  
21 does not occur only locally but along the entire length of the section even at low shear stresses (0.5  
22 N/m<sup>2</sup>). Cigana et al. (2001) investigated the impacts of underflow baffles on the retention of floatables in a  
23 combined sewer system. The study showed that a critical horizontal velocity can develop in overflow  
24 chambers and when this critical velocity is exceeded, floatables that would normally rise to the surface  
25 and be skimmed off are kept in the flow and therefore are not intercepted.

26 Gromaire et al. (2001) documented the sources of pollutants (SS, VSS, COD, BOD<sub>5</sub>, Cd, Cu, Pb, and Zn)  
27 in the Paris combined sewer area called the "Marais" catchment. The erosion of in-sewer pollutants was  
28 the main source of particles and organic matter in wet weather flows, whereas heavy metal loads mainly  
29 originated from runoff due to the corrosion of metallic roofs. A change of the chemical form of heavy  
30 metals was noted during sewer transport. It was hypothesized that a fraction of the dissolved metals from  
31 the runoff adsorbed onto sewer sediments. Griffin et al. (2001) reported on the CSO Control System  
32 Evaluation report for the City of Atlanta, Georgia. Based on the testing, the main pollutants of concern for  
33 meeting water quality standards were copper (Cu), zinc (Zn) and bacteria (fecal coliform). The two metals  
34 exceeded the baseline dissolved criteria frequently enough for remedial measures to be needed.  
35 Supplemental stormwater runoff sampling from parking lots and parks demonstrated high levels of metals  
36 (Cu and Zn) and bacteria. The frequency of exceedences for the stormwater runoff were similar to that of  
37 CSOs; thus, similar water quality compliance issues would remain even if the sewers were fully  
38 separated.

39 Sacramento County, California has completed a two-year project on the impact of grease accumulation in  
40 selected areas of the collection system (Hassey and Joyce 2001). Current grease handling policies of  
41 CSD-1 and local food producing facility wastewater flows result in heavy localized grease accumulation in  
42 the collection system that account for approximately seven percent of the annual grease related sewage  
43 overflows, and 36 percent of the flooded structure mitigation costs related to the overflows.

44

45 **POLLUTION SOURCES**

46 *Atmospheric Sources*

47 Ahn and James (2001) report that atmospheric deposition is a substantial source of phosphorus to the  
48 Florida Everglades. Phosphorus has been measured on a weekly basis since 1974, but the results were  
49 highly variable: the average mean and standard deviation of the calculated P deposition rates for 13 sites  
50 were 41± 33 mg P m<sup>-2</sup> yr<sup>-1</sup>. They found that the atmospheric P deposition load showed high spatial and  
51 temporal variability, with no consistent long-term trend. Because of the random nature of P deposition, the

1 estimated P deposition loads have a significant amount of uncertainty, no matter what type of collection  
2 instrument is used, and replicate sampling is highly recommended. Atasi et al. (2001b) conducted source  
3 monitoring using specialized sampling equipment and ultra-clean analytical methodology to quantify the  
4 concentrations and fluxes of mercury, cadmium, and polychlorinated biphenyl in ambient air, precipitation,  
5 runoff, sanitary sewer, and wastewater treatment plant influent. The relationships between the  
6 atmospheric deposition and runoff on controlled surfaces were also examined. Atmospheric deposition  
7 was found to be the primary source of these pollutants in runoff. They concluded that wet weather flows,  
8 not atmospheric deposition, contributed the main portion of these pollutants to the Detroit Wastewater  
9 Treatment Plant. Atasi et al. (2001a) also pointed out that most water resources regulations (especially  
10 TMDL procedures) do not normally account for atmospheric deposition sources. Tsai et al. (2001)  
11 described their pilot study, conducted from August 1999 through August 2000, that estimated the loading  
12 of heavy metals from the atmosphere to San Francisco Bay. Dry deposition flux of copper, nickel,  
13 cadmium, and chromium was approximately 1100 +/- 73, 600 +/- 35, 22 +/- 15, and 1300 +/- 90  
14  $\mu\text{g}/\text{m}^2/\text{year}$ , respectively. The volume-weighted average concentrations of these trace metals in the rain  
15 water were 1.2, 0.4, 0.1, and 0.2  $\mu\text{g}/\text{L}$ , respectively. Direct atmospheric deposition onto Bay waters, from  
16 both dry deposition and rainfall, contributed approximately 1900, 930, 93 and 1600 kg/yr of copper, nickel,  
17 cadmium and chromium, respectively. Stormwater runoff contributed approximately twice as much as the  
18 loading from direct atmospheric deposition. Direct atmospheric deposition was therefore found to be a  
19 minor contributor to the total load of these pollutants to the Bay.

#### 20 *Roof Runoff*

21 Wallinder and Laygraf (2001) studied the seasonal variations in the corrosion and runoff rates from  
22 copper roofs. Their experiments lasted for two years, at one urban and one rural location. Seasonal  
23 variations in corrosion rates were observed at the rural site, likely associated with variations in humidity,  
24 while no seasonal variations were observed at the urban site. The corrosion rates continually decreased  
25 with time. The yearly copper runoff rates ranged from 1.1 to 1.7  $\text{g m}^{-2}\text{y}^{-1}$  for the urban site, and from 0.6 to  
26 1.0  $\text{g m}^{-2}\text{y}^{-1}$  for the rural site. The runoff rates were significantly lower than the measured corrosion rates  
27 as long as the adhering copper patina was increasing with exposure time. From 70 to 90% of the copper  
28 in the runoff (collected immediately after leaving the surface) was present in the most bioavailable form,  
29 the hydrated cupric ion,  $\text{Cu}(\text{H}_2\text{O})_6^{2+}$ .

#### 30 *Highway and Road Runoff*

31 Glenn et al. (2001a and 2001b) described their research at highway test sites in Cincinnati, Ohio  
32 investigating the effects of traffic activities and winter maintenance on the behavior of particulates in the  
33 runoff. They found that urban snow has a much greater capacity to accumulate traffic-related pollutants,  
34 as compared to stormwater, due to longer residence times before melting, and the snow's porous matrix.  
35 Parameters such as residence time, solids loadings, alkalinity, hardness and pH influence the heavy  
36 metal partitioning in the snow. They found that Pb, Cu, Cd, Zn, Al, Mg, and Fe were mostly particulate  
37 bound, while Na and Ca were mostly dissolved. Partition coefficients for most heavy metals in snowmelt  
38 water ranged from 103 to 106 L/kg. Stenstrom et al. (2001) studied freeway runoff from three sites in the  
39 west Los Angeles area. Each site was sampled for 14 storms during the 1999-2000 rainy season.  
40 Samples were collected very early in the storm in order to compare water quality from the first runoff to  
41 water quality from the middle of the storm. A large range of water quality parameters and metals were  
42 analyzed. The data showed large first flushes in concentration and moderate first flushes in mass  
43 emission rates. Zhou et al. (2001) studied accumulations of heavy metals in roadside soils. Heavy metal  
44 accretion in the surficial soils was a function of depth, surface drainage patterns, distance from the  
45 pavement edge and soil indices. Rapid decreases in heavy metal accumulations were found as the  
46 distance from the pavement increased. Plasticity and organic matter content were important soil  
47 characteristics affecting the observed heavy metal concentrations.

#### 48 *Wastewater, CSOs and SSOs*

49 The Massachusetts Water Resources Authority (MWRA) completed its Long-Term Control Plan for CSO  
50 control in 1997. The plan used a watershed approach to assess the impacts of CSOs in relation to other  
51 sources of pollution in the watershed, including stormwater and upstream flows. The EPA and the  
52 Massachusetts Department of Environmental Protection agreed to revise the water quality standards for  
53 certain waters within Boston Harbor. For the Charles River, a two-year water quality variance was granted

1 to allow further study of the impacts of non-CSO sources of pollution, as well as for additional CSO  
2 control alternatives. Important current activities described by Walker et al. (2001) include an assessment  
3 of additional CSO storage; performance evaluations of the MWRA's Cottage Farm CSO facility;  
4 development of a more detailed stormwater runoff model for areas tributary to the Charles River; and  
5 upgrading the receiving water model for the Charles River.

#### 6 *Industrial Sources*

7 Chang and Duke (2001b) summarized their research that examined stormwater quality from eight auto  
8 dismantling facilities in Los Angeles, CA, over a three year period. The majority of the samples had  
9 constituents exceeding stormwater discharge guidelines, but were highly variable. Talbot (2001)  
10 discussed some of the stormwater management challenges and solutions for power generation facilities.

11 Stormwater from a modern waste disposal site was characterized by Marques and Hogland (2001). They  
12 examined 22 different constituents and found that suspended solids, COD, BOD, total nitrogen and total  
13 phosphorus, exceeded discharge standards. Some of the runoff constituents were greater than found in  
14 the leachate from covered landfill sites. Copper, zinc and nickel were the most commonly detected heavy  
15 metals, being found in every sample. Surprisingly, the concentrations of zinc, nickel, cobalt, iron and  
16 cadmium found in runoff from composting areas were greater than the metal concentrations found in the  
17 runoff from areas having stored and exposed scrap metal. They concluded the presence of large amounts  
18 of organic compounds, plus site specific drainage pathways, were responsible for these findings.

#### 19 *Specific Pollutant Sources*

20 Nonpoint sources of heavy metals (Hg, Cd, Cu, Zn, Pb, Ni) in the Rhine River watershed were  
21 investigated by Mohaupt et al. (2001). They found that urban stormwater was the most important source  
22 for these metals in the Dutch portion of the watershed. Erosion was of lesser importance, while  
23 atmospheric deposition onto open water was a much less important source. Anthropogenic nonpoint  
24 sources accounted for 40 to 80% of the total sources for some metals. They recommended further  
25 studies of urban nonpoint sources of heavy metals and on ways to improve urban stormwater  
26 management. A mass balance of all known sources and sinks for heavy metals (Ag, Cd, Cu, and Pb) in  
27 New Haven Harbor, CT, was conducted by Rozan and Benoit (2001). Sources included direct  
28 atmospheric deposition, rivers, treated sewage effluent, combined sewer overflows, and permitted  
29 industrial discharges. All of the fluxes were directly measured, and the uncertainties were quantified.  
30 River inputs accounted for most of the total yearly metal discharges, while the salt marshes removed  
31 about 20 to 30% of the metals from the rivers before reaching the harbor. Atmospheric deposition is of  
32 minor importance, and is comparable to sewage effluent discharges. Davis et al. (2001a) presented  
33 loading estimates of lead, copper, cadmium, and zinc in stormwater from different sources. They  
34 reviewed available data from the literature, and conducted controlled experiments and other sampling.  
35 Specific sources that they examined included building siding and roofs; automobile brakes, tires, and oil  
36 leakage; and wet and dry atmospheric deposition. The most important sources they identified were  
37 building siding for all four metals, vehicle brake emissions for copper, and tire wear for zinc. Atmospheric  
38 deposition was an important source for cadmium, copper, and lead.

39 Ellis and Chatfield (2001) reported that nonpoint discharges of oil and hydrocarbons to urban receiving  
40 waters constitutes a major pollution source, being responsible for up to 30% of all reported water pollution  
41 incidents in the UK. Commercial and industrial areas, along with heavily used highways, are the major  
42 sources of these pollutants, with about 20,000 to 24,000 tonnes of oil per year being discharged to urban  
43 receiving waters from these sources. Blanchard et al. (2001) investigated PAHs and PCBs in the  
44 combined urban flows to the Acheres wastewater treatment plant in Paris, France, under different  
45 meteorological conditions. The PAH concentrations were found to increase much more than the PCB  
46 concentrations during wet weather. They concluded that atmospheric wet deposition was the most  
47 important source for the PCBs, while urban stormwater was the most important source for the PAHs.

48 The suspected source of the bacterial contamination that has affected Lake Pontchartrain, New Orleans,  
49 LA, is urban stormwater that is collected and pumped to the lake. Barbe' et al. (2001) concluded that the  
50 stormwater may be contaminated by sanitary sewage. Five monitoring stations were selected for study  
51 that had varying distances to stormwater drainage canals. Fecal coliform concentrations was found to be  
52 wet weather-dependent at all stations, except for one. They found that the water is generally unsuitable

1 for primary contact recreation, especially close to urban drainage canals, for at least two to three days  
2 following a storm event. The study period was characterized by unusually low rainfall and lower than  
3 typical fecal coliform observations. They suspected that the lower bacteria concentrations may have been  
4 due to the drought conditions, rather than from decreased contamination. Brion et al. (2001) addressed  
5 sources of bacteria indicators. They found that first flush discharges contained high levels of pathogens,  
6 originating from storm sewers, overflowing domestic sewers, and surface scour. They present some new  
7 analytical tools (bacterial ratio that varies with age) and modeling tools (neural network) to help identify  
8 the predominant bacteria sources in stormwater.

## 10 **MONITORING AND SAMPLING**

### 11 *General*

12 The Patrick Center for Environmental Research of The Academy of Natural Sciences in conjunction with  
13 the University of Delaware developed a method to assess streams based on geomorphic, habitat and  
14 riparian features (Cianfrani et al., 2001). An overall Stream Quality Index (SQI) was developed that could  
15 classify streams into four major categories: severely impaired, impaired, moderately impaired, and slightly  
16 or non-impaired. The SQI rating was used with other natural-resource inventory data in restoration  
17 planning and overall management of the Fairmount Park system.

### 18 *Rainfall and Flow Measurement*

19 The Tropical Rainfall Measuring Mission (TRMM) is a United States-Japan joint project to measure rainfall  
20 from space (Ohsaki 2001). In this paper, rain/no-rain discrimination for the TRMM was validated through  
21 simulation and theory for the no-rain condition and by comparison with the ground-based radar data for  
22 rain conditions. The precipitation radar (PR) aboard the TRMM demonstrated the feasibility of measuring  
23 rainfall from space (Sadowy et al., 2001). The Second Generation Precipitation Radar (PR-2) has been  
24 developed. The PR-2 will improve capabilities and substantially reduce system mass compared to the  
25 TRMM PR. This paper surveyed the technologies for the PR-2.

26 A combined Microwave/Infrared Rain Rate Algorithm (MIRRA) was presented by Miller et al. (2001).  
27 MIRRA combined the strengths of these two broad approaches to rain rate measurement from space.  
28 The algorithm was tested using data from the TOGA-COARE campaign, with shipboard radar rain rate  
29 estimates used as truth. Results indicate enhanced performance in bias, correlation and rms error for  
30 MIRRA. Calibrated radar rainfall systems, often called 'virtual rain gages,' have become popular due to  
31 the advantages they offer over conventional precipitation monitoring networks (Orie et al., 2001). The 3  
32 Rivers (Allegheny County, Pennsylvania) Wet Weather Demonstration Program made virtual rain gauges  
33 available to the public by integrating the calibrated rainfall system with the Internet. A methodology was  
34 developed that matched coincident space-based radar (PR) and ground-based volume scanning weather  
35 radar observations in a parallel three-dimensional Cartesian grid (Anagnostou et al., 2001). The data  
36 matching was performed to minimize uncertainties associated with the type of weather seen by the  
37 radars, grid resolution, and differences in radar sensitivities, sampling volumes, viewing angles, and radar  
38 frequencies. The authors compared reflectivity observations from the PR and several U.S. weather  
39 surveillance Doppler radars (WSR-88D) as well as research radars from the TRMM field campaigns in  
40 Kwajalein Atoll and the Large Biosphere Atmospheric (LBA) Experiment. Correlation values above 0.8  
41 were seen between PR and ground radar.

42 A daily rainfall disaggregation method that yields regional hourly rainfall estimates was presented in terms  
43 of application to continuous-simulation watershed models (Socolofsky et al., 2001). When compared with  
44 hourly data, the method reproduced well the variance, lag-1 autocovariance, and probability of zero  
45 rainfall. Application of the method in the upper Charles River watershed in eastern Massachusetts  
46 demonstrated that the method greatly improved the river flow simulation capability of the hydrologic model  
47 over alternate simulations using hourly rainfall from outside the watershed. An overview of weather radar-  
48 related developments in South Africa, as related to hydrological research and operations, was given by  
49 Terblanche et al. (2001). This program developed the necessary infrastructure, expertise and related  
50 hardware and software to collect and archive high-quality radar data. Shortcomings of weather radar data  
51 for hydrological applications were described and the ongoing research efforts to address these  
52 shortcomings were highlighted. Valeo and Tang (2001) developed a model for the second order process

1 of rainfall in an area of southern Ontario. Hourly rainfall data collected from 17 gages over a period of five  
2 years were used to develop a correlation function for rainfall, with an exponential model for the correlation  
3 function being selected over a spherical and Gaussian model. A regional, index-storm approach based on  
4 L-moments was applied to estimate short duration (less than or equal 24 h) design rainfalls in South  
5 Africa (Smithers and Schulze 2001). The regionalization was performed using only site characteristics  
6 (latitude, longitude, altitude, concentration of precipitation, mean annual precipitation (MAP), rainfall  
7 seasonality and distance from sea). The index storm used was the mean of the annual maximum series  
8 and a relationship was derived between the index storm and MAP.

9 Rainfall simulation experiments were carried out on more than 40 1-m<sup>2</sup> plots to measure infiltration point-  
10 processes (Perrin et al., 2001b). The high density of measurement devices allowed identification of the  
11 origin and nature of the various contributions to runoff for the different physiographic units of the  
12 watershed: urban area, farmland, pasture, forested land, and paramo. Variations in the runoff coefficients  
13 were related first to the baseflow and second to the amount of rainfall in the previous 24 h. In the paper  
14 by Morin et al. (2001), high-resolution meteorological radar data are used for the determination of a  
15 characteristic temporal scale for the hydrological response of the basin – the ‘response time scale’ (T-s\*).  
16 The identification of the response time scale was carried out by analysis of observations only and without  
17 assuming a specific rainfall-runoff model. For all analyzed basins a stable response time scale was  
18 identified with relatively short time scales found for the urban and arid basins (15-30 min) and longer time  
19 scales for the rural basins (90-180 min). Habib and Krajewski (2001) reported on the application of a  
20 computational fluid dynamics-based method to improve the aerodynamic design of rainfall measurement  
21 devices, including a two-dimensional video disdrometer that provides information about raindrop size  
22 distribution. The distorted wind field around and inside the instrument's body was simulated using a three-  
23 dimensional numerical model. Raindrop trajectories were simulated to investigate the wind effect on the  
24 catchment efficiency of the instrument.

25 The study by Kurz and Qualls (2001) evaluated the EPA's PCS (Permit Compliance Study) database and  
26 determined the various parameters that may be useful in developing I/I control and sewer rehabilitation  
27 strategies on a state and regional level. The study summarized DMRs (monthly NPDES Discharge  
28 Monitoring Reports) for 1999 from 790 systems in Region 4 categorized as “major.” Maheepala et al.  
29 (2001) described the issues that need to be considered when conducting a successful and cost-effective  
30 hydrologic data monitoring program. It was found that that tipping bucket resolutions up to 0.5 mm would  
31 give reasonably accurate results in urban stormwater modeling, and two-minute data logging intervals  
32 were suitable for flow data monitoring. The results also suggested that combining low cost simple flow  
33 measurements and limited high cost sophisticated measurements could reduce the data acquisition cost  
34 without compromising the accuracy of flow hydrographs measured in stormwater conduits. Remote  
35 sensing was used as a source of data to address the spatial variability of hydrologic processes such as  
36 storm runoff (Melesse et al., 2001). Remote sensing provided quantitative land cover information of  
37 suitable spatial resolution for model inputs. The study area was the S-65A sub-basin of the Kissimmee  
38 River basin in south Florida, and results showed that the temporal and spatial variability of runoff volume  
39 resulting from changes in land cover could be determined from Landsat images.

40 The WERF research described by Schultz et al. (2001b) will assist municipal agencies to quantify and  
41 simulate rainfall-derived infiltration and inflow (RDII) that affects their sewer systems. Eight major  
42 categories of RDII analysis methods were tested in several diverse sewersheds and metrics suitable for  
43 objective comparison of the RDII analyses were identified. An accurate dye dilution testing protocol using  
44 Rhodamine WT was used to quantify flow meter accuracy in the Greater Detroit Regional Sewer System  
45 (Stonehouse et al., 2001). The seven technologies assessed were electromagnetic induction meters  
46 (magmeters); full-conduit, multiple-path, transit-time meters; full-conduit, open-channel, ultrasonic meters;  
47 flumes; and weirs. It was concluded that (i) there were observable accuracy differences between flow  
48 meter technologies, (ii) objective standards like dye dilution testing were critical, (iii) verifying installed  
49 accuracy was important, and (iv) the simplest technology that can be used often was the best. Swarner et  
50 al. (2001b) reported on the effort by King County determine the wet weather performance and geographic  
51 distribution of I/I through its entire service area; conduct several pilot rehabilitation projects to evaluation  
52 rehabilitation effectiveness; develop and calibrate an accurate hydraulic model of the system; and  
53 implement a Regional I/I Control Program. The paper discussed the political and administrative actions to  
54 achieve consensus among the local agencies and analyzed the I/I results as a function of basin size.

1     *Toxicity Testing*

2     Bdour et al. (2001) examined the relationships between Index of Fish Density (IFD) and man-made  
3     disturbances, watershed parameters, habitat parameters, water quality parameter, and climate  
4     parameters for the 61 HUCs (Hydrological Unit Code) for the South Fork Clearwater River basin in Idaho  
5     and 15-20 reaches for Red River and Newsome Creek within the South Fork Clearwater River Basin. The  
6     results showed that incorporating both fine and coarse scale parameters in any assessment model will  
7     improve the correlations. Biomonitoring using juvenile giant clams (*Tridacna maxima*) was conducted in  
8     conjunction with sediment and stormwater sampling at U.S. Army Kwajalein Atoll. Elevated levels of  
9     metals, PAHs, pesticides, and PCBs were detected in clams from the sampling locations. Trends in clam  
10    tissue, sediment and stormwater data suggested that both sediment and stormwater are significant  
11    sources of the pollutants detected in clams.

12    *Testing for Chemical Pollution and Pollution Tracers*

13    Automated samplers were used to collect urban runoff in Pecan Creek, Denton, Texas (Appel and Hudak  
14    2001). The results showed that concentrations of some heavy metals (cadmium and arsenic), as well as  
15    pesticides (atrazine and diazinon), were within ranges typical for urban runoff and first flush samples were  
16    more concentrated than composite samples for most of the constituents. Automated samplers were seen  
17    to be an effective way to sample urban runoff in Pecan Creek. Ruban et al. (2001) studied the SS and  
18    COD concentrations in combined sewers using in-line ultraviolet and visible absorbances. The optical  
19    measurement of SS and COD could then be used to determine average or long term pollution loads, for  
20    example the yearly impact of urban stormwater discharges. Continuous and on-line measurements would  
21    allow agencies to react with short delay to unexpected phenomena. A portable, field rugged, sampling  
22    and analysis system was developed for the rapid screening of aqueous samples during scoping and  
23    remediation studies (Beals et al., 2001). The equipment uses ion selective solid phase extraction (SPE)  
24    disks and counted for the radionuclide of interest in the field using portable detectors. The Savannah  
25    River Site H-area tank farm storm water runoff system was analyzed for Sr-90 and Cs-137.

26    The investigation by Moustafa and Havens (2001) determined the effect of sampling frequency and  
27    sampling type on estimates of monthly nutrient loads and flow-weighted nutrient concentrations in a  
28    constructed wetland. The results showed that bi-weekly composite sampling could be used to monitor  
29    nutrient concentrations and loads discharged from larger-scale Everglades Stormwater Treatment Areas  
30    (STAs) now under construction. Compounds including coprostanol, epicoprostanol, cholesterol and  
31    dihydrocholesterol were quantified in CSO waters and their relative ratios calculated to investigate their  
32    potential application as source tracers (Marvin et al., 2001). A mean coprostanol:epicoprostanol ratio of  
33    approximately 70 was calculated for CSOs. When combined with physical measurements (e.g., current  
34    velocities) and meteorological data, these ratios may assist in determining the influence of shore-based  
35    activities, including sewage treatment and livestock operations, on aquatic systems. Indicators for  
36    assessing pathogen contamination and for distinguishing human origin and animal origin were  
37    investigated (Sankararamkrishnan and Guo (2001) with samples being taken from storm sewer outfalls  
38    to a coastal lake during wet weather as well as from the lake during the dry weather. The FC-to-FS ratio  
39    indicated the contribution of both human and animal wastes, with the presence of human wastes  
40    confirmed by increased concentrations of caffeine, fluorescence whitening agent, fluoride and anionic  
41    surfactant during wet weather.

42    *Biological and Microbiological Testing*

43    Elevated bacteria levels indicated significant impairment to surface water quality in many urban areas  
44    within the Great Lakes watershed (Murray et al., 2001a). CSO outfalls were assumed to be the major  
45    source of bacteria to streams in many of these urban areas, including the Rouge River of southeastern  
46    Michigan. The data depicted a strong influence of upstream water and rural runoff on the water quality of  
47    the Rouge River, and FC to FS ratios suggested the primary source of bacteria throughout the watershed  
48    was from domesticated animals and wildlife and not from CSOs. Sources of the indicator bacteria total  
49    coliform, fecal coliform, and enterococcus were investigated in stormwater flows discharging to Mission  
50    Bay in San Diego, California (Schiff and Kinney 2001). Stormwater flows were targeted because long-  
51    term monitoring had indicated that wet weather discharges were the predominant source of bacterial  
52    contamination. Upstream tracking during multiple storm events on two watersheds showed that bacterial  
53    sources were diffuse and widespread. Two case studies were used to demonstrate how WERF's

1 ecological risk assessment methods, Aquatic Ecological Risk Assessment: A Multi-Tiered Approach,  
2 could be used to improve the ecological- and cost-effectiveness of water quality management (WERF  
3 2001b). The two cases (Salado Creek in the City of San Antonio, Texas, and the Jordan River in Utah)  
4 were explored in detail, including direct and indirect effects of changes in flow on fish and invertebrate  
5 aquatic life.

## 7 **SURFACE WATER IMPACTS**

### 8 *Flow Regime Impacts*

9 Rose and Peters (2001) reported an investigation that examined streamflow characteristics that changed  
10 during the period from 1958 to 1996, in a highly urbanized watershed (Peachtree Creek) compared to  
11 less-urbanized watersheds and non-urbanized watersheds, in the vicinity of Atlanta, GA. Data were  
12 obtained from seven US Geological Survey stream gauges, 17 National Weather Service rain gauges,  
13 and five USGS monitoring wells. The fraction of the rainfall occurring as runoff in the urban watershed  
14 was not significantly greater than for the less-urbanized watersheds, but this ratio did decrease from the  
15 higher elevation and higher relief watersheds to the lower elevation and lower relief watersheds. For the  
16 25 largest stormflows, the peak flows for the urban creek were 30% to 100% greater than the peak flows  
17 in the streams located in the less developed areas. The streamflow also decreased more rapidly after  
18 storms in the urban stream than for the other streams. The low flow in the urban creek was from 25 to  
19 35% less than for the less developed streams, likely caused by decreased infiltration due to the more  
20 efficient routing of stormwater and the paving of groundwater recharge areas.

21 Weng (2001) describes how GIS was used in conjunction with distributed hydrological modeling. GIS was  
22 able to document the changing spatial patterns of urban growth in the Zhujiang Delta of southern China.  
23 Extensive urban growth over the past two decades has created severe problems in water resources  
24 management, indicated by an increase in annual runoff depth during the 1989-1997 period. The  
25 urbanization lowered potential maximum storage, and increased runoff.

### 26 *Erosion, Channel Stability, and Sediment*

27 An emerging concept in channel design uses sediment transport as the basis for quantifying channel  
28 stability. Byars and Kelly (2001) examined channel stability in the Austin, TX, area. They concluded that a  
29 channel that is not undergoing excessive erosion or sedimentation has a function and form similar to a  
30 natural stream and should be the goal of good channel design. This approach, however, requires a more  
31 comprehensive understanding of the local climate, geology, hydrology, and stream mechanics than  
32 historically utilized.

33 Hunt and Grow (2001) describe a field study conducted to determine the qualitative and quantitative  
34 impact to a stream from a poorly controlled construction site. They used fish electroshocking, Qualitative  
35 Habitat Evaluation Index, and zigzag pebble count studies. The 33 acre construction site consisted of  
36 severely eroded silt and clay loam subsoil and was located within the Turkey Creek drainage, Scioto  
37 County, OH. The number of fish species declined (26 to 19) and the number of fish found (525 to 230)  
38 decreased significantly when comparing upstream unimpacted reaches to areas below the heavily  
39 eroding site. The Index of Biotic Integrity and the Modified Index of Well-Being, common fisheries indexes  
40 for stream quality, were therefore reduced from 46 to 32 and 8.3 to 6.3, respectively. Upstream of the  
41 area of impact, Turkey Creek had the highest water quality designation available (Exceptional Warm  
42 Water Habitat); but fell to the lowest water quality designation (Limited Resource Water) in the area of the  
43 construction activity. Water quality chemical analyses conducted on samples from upstream and  
44 downstream sites verified that these impacts were not from chemical affects alone.

### 45 *Biological Impacts*

46 Horwitz et al. (2001) examined the different fish communities in paired reaches of streams having  
47 forested vs. unforested riparian zones. The streams traversed land uses ranging from rural to highly  
48 urban. Their preliminary results found that the type of buffer had little effect on the fish communities in the  
49 rural reaches. However, the fish community patterns were much more variable in the urban reaches,  
50 being affected by the effects of urbanization on channel morphology, habitat, hydrology, and water  
51 quality. Sonneman et al. (2001) studied the effects of urbanization on the benthic diatom communities in

1 streams near Melbourne, Australia. The subcatchments had imperviousness levels ranging from 0 to  
2 51%. They found that the differences observed were best explained by variations in nutrient  
3 concentrations (phosphorus, ammonia, and total nitrogen). The level of urbanization, along with the  
4 presence of small sewage treatment plants at a few sites, influenced the nutrient concentrations found.  
5 They observed that diatoms were better indicators of nutrient enrichment, while macroinvertebrates were  
6 better indicators of catchment disturbance. Walsh et al. (2001) reported the effects of macroinvertebrates  
7 at the same Melbourne area locations as reported by Sonnenman et al.,(2001). Responses were more  
8 obvious in the eastern areas (having a wide range of development) and were associated with  
9 conductivity, while the western areas (having little development) had little observed variations. The  
10 metropolitan areas were all severely degraded, having high abundances of a few tolerant taxa. Extensive  
11 development of urban drainage systems, even in areas having low urban densities, increased the  
12 observed degradation.

13 Fifty et al. (2001) investigated the impact of wastewater and stormwater discharges on the coastal  
14 receiving waters on a coral atoll in the middle of the Pacific Ocean (Kwajalein Atoll), a pristine marine  
15 environment. They used *Tridacna maxima*, a giant clam species, as an indicator organism. Juvenile  
16 clams were deployed for a 3-month period in the vicinity of pollutant sources and reference sites, and  
17 then analyzed for metals, PAHs, pesticides, and PCBs. The clams were a successful bioindicator, with  
18 tissue samples from sites near pollutant sources (contaminated sediment and stormwater) containing  
19 higher concentrations of chemicals than at the reference sites.

20 Grant et al. (2001) reported frequent elevated levels of enterococci bacteria in the surf zone at Huntington  
21 Beach in southern California. They studied the sources of these indicators and found high levels in urban  
22 runoff, bird feces, marsh sediments, and on marine vegetation. They concluded that urban runoff had  
23 relatively little impact on these elevated surf zone bacteria levels because of the long travel time needed  
24 for the urban runoff to travel from the source areas to the ocean. They found that marsh sources were  
25 more likely responsible for the high surf zone bacteria levels. Jiang et al. (2001) used a nested-PCR  
26 analytical method to detect viruses in coastal waters at 12 beach locations in Southern California. The  
27 sampling locations were all impacted by urban runoff sources. The sampling locations were all located at  
28 the mouths of major rivers and creeks. Human adenoviruses were detected in 4 of the 12 samples (880 to  
29 7,500 per L), coliphages were found in all 12 samples (5.3 to 3330 PFU/L), and F-specific coliphages  
30 were found in 5 of the 12 samples (5.5 to 300 PFU/L). The bacterial indicator levels (total coliforms, fecal  
31 coliforms, and enterococci) all exceeded the California recreational water quality limits, but the bacteria  
32 levels found did not correlate well with the observed human adenovirus levels. They concluded that the  
33 standards that rely on bacteria observations to indicate viral quality of recreational waters be re-  
34 evaluated, and that more routine monitoring of human viruses be conducted on a regular basis.

35 Mahin (2001) reviewed recently completed epidemiological studies conducted after the publication of the  
36 1986 EPA bacteria guidance. The Massachusetts Dept. of Environmental Protection were concerned  
37 about the risk level that is associated with stormwater runoff to recreational waters, and if enterococci or  
38 *E. coli* can adequately predict the range of possible illnesses that may affect swimmers in contaminated  
39 marine waters. Crowther et al. (2001) examined the relationships between microbial water quality and  
40 environmental conditions in coastal recreational waters along the Fylde coast, UK. Eight designated  
41 bathing beaches continued to exhibit unreliable compliance with the Imperative standards for total  
42 coliforms and fecal coliforms, despite significant reductions in geometric mean concentrations following  
43 major improvements in the sewerage infrastructure. Fecal streptococci concentrations have remained  
44 high. Before the improvements, higher bacterial concentrations were strongly associated with rainfall; and  
45 sewage sources were important for TC and FC, but less important for FS. Since the improvements,  
46 catchment sources seem to be of greater importance. Pendleton (2001) reported that despite posted  
47 warnings and educational campaigns warning about the health risks associated with storm water  
48 pollution, swimmers continue to swim in Southern CA coastal areas polluted by stormwater. Passive  
49 means of preventing exposure to marine pollution (e.g., posted signs) were found to be more effective if  
50 combined with the active management of other beach amenities. Pendleton et al. (2001) further found  
51 that despite documented successes in the battle to clean up the coastal waters of Southern California,  
52 Los Angeles County residents continue to view the ocean more as a place of pollution than a healthy  
53 place for bathing and swimming. Survey results suggest that perceptions of coastal water quality may be  
54 influenced less by "current coastal education campaigns" and more by the media and other factors.

1 *Chemical Impacts*

2 Diamond et al. (2001) found that the effects of toxicants depended on a combination of both chemical and  
3 flow characteristics. Conventional laboratory testing of toxicants with constant exposure concentrations  
4 are not very applicable to wet weather flow conditions. They surveyed more than 30 toxicological  
5 investigations that have used time-dose or pulsed/intermittent exposure, 15 contaminants, and 10  
6 different species. They concluded that it is possible to predict the chronic effects of fluctuating exposures  
7 of fast-acting contaminants (such as ammonia or sodium chloride) using available acute toxicological  
8 models. Wong et al. (2001) developed a monitoring method using the green alga *Selenastrum*  
9 *capricornutum* to predict levels of heavy metals in water. When exposed to stormwater samples, the  
10 specific activity of the peroxidase in the cell extract was directly related to the copper and lead  
11 concentrations. The peroxidase responses were also correlated with the 96 hr biomass toxicity assay of  
12 *S. capricornutum*. They concluded that the use of this peroxidase can be used as a marker for testing  
13 heavy metal toxicity in the water.

14 *Habitat Management and Restoration*

15 The purpose of the EPA-funded Arid West Water Quality Research Project is to conduct scientific  
16 research to develop appropriate water quality criteria and improve the scientific basis for regulating  
17 wastewater and stormwater discharges in the arid and semi-arid West. Meyerhoff et al. (2001) described  
18 a study where historical and site reconnaissance level data were gathered on ten effluent dependent  
19 waters. They found that while the aquatic community may be limited, significant benefits occurred in the  
20 terrestrial communities that developed in response to the created aquatic environment. Based on the  
21 concerns raised during this research, the EPA's Region IX published a "net ecological benefit" guidance  
22 document that provided an opportunity to recognize the non-aquatic benefits gained from effluent  
23 discharged to otherwise dry riverbeds.

24 *Environmental Effects of CSO and SSO*

25 Borchardt and Reichert (2001) describe their study of the River Lahn, a moderately polluted 5th order  
26 stream in Germany for which the connectivity of surface/subsurface flows and mass fluxes within river  
27 sediments have been intensively investigated. The hyporheic flow between a downwelling and upwelling  
28 zone of a riffle-pool reach of the river was studied using tracers and continuous records of water chemical  
29 characteristics. High diurnal fluctuations of oxygen traveled to considerable depths in the sediment and  
30 oxygen levels in the interstitial water decreased considerably while traveling through the riffle reach. The  
31 resulting model is being used to examine the effects of CSO discharges. They found that CSOs may  
32 cause anoxic sediment oxygen conditions for extended periods of time. Michels (2001) described a CSO  
33 Basin study conducted on the Menominee River, MI, to see if the facility met the Michigan water quality  
34 standards and if it provided adequate treatment. Four sets of samples were collected upstream and  
35 downstream of the facility for comparison during discharge events. They found that the water quality  
36 standards were not violated during overflow discharges to the Menominee River.

37 *Risk Assessment*

38 Bosley et al. (2001) described a study on the Upper Roanoke River Watershed (URRW) in southwest  
39 Virginia, using HSPF to evaluate the hydrologic impacts of land use change. Continuous simulations were  
40 conducted to investigate the hydrologic effects of various spatial arrangements of development in  
41 residential areas, with and without networks of primary and secondary roads. Beckers and Frind (2001)  
42 used a steady-state stormwater model of the Oro Moraine aquifer in Ontario to examine long-term  
43 temporal variations in the flow regime associated with changes in aquifer recharge. They found that  
44 nearby urban development would have a significant impact on the baseflow to the swamp, in addition to  
45 baseflow impacts to nearby local streams. The model also is being used as a guide to future data  
46 collection in the area.

47

48 **GROUNDWATER IMPACTS**

49 *Planned Groundwater Recharge*

1 A water balance model was developed for a semiarid landscape of Spain (Bellot et al., 2001). The  
2 components were the soil water content, the actual evapotranspiration (Eta), and both the aquifer  
3 recharge (deep drainage) and runoff, which reflected water recharge, human use and soil erosion  
4 impacts. Combining the model predictions with the land cover vegetation units on the aquifer recharge  
5 area, the effects of some management policies on the deep drainage and runoff in five simulation  
6 scenarios were compared. Extreme precipitation during a wet year led to a higher erosion risk. Well  
7 clogging will impede the use of aquifer storage and recovery (ASR) wells (Dillon et al., 2001). In this  
8 study, the Membrane Filtration Index (MFI), a standard test of the rate at which water clogs a membrane  
9 filter, has been used with turbid and organic-rich waters. Waters from 12 sites (mains, urban stormwater  
10 and reclaimed water) which are or have the potential to be water sources for aquifer storage and recovery  
11 (ASR) in southern Australia, were analyzed for MFI, turbidity, total suspended solids, total organic carbon,  
12 particle size and SEM. Little more than half the variance in MFI could be explained with the measured  
13 water quality parameters, likely due to the complex nature of the inorganic and organic particles present  
14 in the waters. The ongoing field study has been designed to determine if MFI can be related to well  
15 clogging at one of the focus sites.

16

## 17 **DECISION-SUPPORT SYSTEMS**

### 18 *Numerical Models*

19 Rainfall analyses. Aerial variations in rainfall were investigated by Balascio (2001), using unbiased  
20 multiquadric analyses to reduce the problem of negative rain gauge weighting when determining the  
21 representative rain depth for an area. Willems (2001) developed a stochastic spatial rainfall generator for  
22 use in small urban catchments. For applications at small scales, the individual rain cells need detailed  
23 descriptions. Data from a dense network of rain gauges at Antwerp were used for verification of the  
24 process. Cameron et al. (2001) successfully modeled extreme rainfalls using a generalized Pareto  
25 distribution to represent the rain depths of high intensity rain cells. Trefryl et al. (2001) used a partial  
26 duration series/ generalized Pareto regional index-flood procedure for updating rainfall intensity-duration-  
27 frequency estimates for the State of Michigan. Porras and Porras (2001) examined all series of extreme  
28 rainfall depths to produce less ambiguous depth-duration-frequency and intensity-duration-frequency  
29 curves for Venezuela. Cheng et al. (2001) used a simple scaling property of rainfall to guarantee that the  
30 normalized rainfall rates of different storm durations are identically distributed. They proposed a  
31 nonstationary Gauss-Markov model, based on the annual maximum events that arise from the dominant  
32 storm type to obtain the most likely hyetograph. This method allows translating hyetographs between  
33 storms of different durations. Koutsoyiannis and Onof (2001) developed a rainfall disaggregation method  
34 using adjusting procedures on a Poisson cluster model. This method allows the possible extension of a  
35 short hourly time-series with the use of longer-term daily rain data.

36 Rainfall – runoff modeling parameter estimation. Bates and Campbell (2001) and Campbell and Bates  
37 (2001) used a Markov chain Monte Carlo procedure to select rainfall – runoff model parameters.  
38 Regionalization of the model parameters was accomplished for all parameters simultaneously via a  
39 regional function that links the posterior means to watershed characteristics. They demonstrated the  
40 methodology using an eight-parameter conceptual rainfall-runoff model and two case studies from  
41 southeastern Australia. Perrin et al. (2001a) questioned whether the use of large numbers of modeling  
42 parameters improve model performance. They conducted an extensive comparative assessment using 19  
43 lumped models on 429 catchments, mostly in France, and with some located in the United States,  
44 Australia, the Ivory Coast, and Brazil. They found that the bulk treatment of the data showed that the  
45 complex models outperformed the simple models in the calibration mode, but not in the verification mode.  
46 They concluded that some simple models can yield promising results, although they are not able to  
47 handle all types of problems. Kokkonen and Jakeman (2001) also found that a model with less  
48 conceptualization could provide a more accurate reproduction of streamflow for some situations. They  
49 conclude that the more complexity one wants to include in the model structure, the more types of data  
50 and higher information content are required. When only rainfall-runoff data are available, it is difficult to  
51 justify substantial conceptualization of complex processes. Wagener et al. (2001) state that many existing  
52 hydrological models do not make the best use of available information, resulting in additional  
53 uncertainties in model structure and parameters, and a lack of detailed information regarding model

1 behavior. They propose a framework for appropriate levels of model complexity as a function of available  
2 data, hydrological system characteristics, and modeling purpose.

3 Liong et al. (2001) utilized a Pareto front, or trade-off curve, to obtain the best overall calibration for  
4 different conditions. Ndiritu and Daniell (2001) used an improved genetic algorithm for rainfall-runoff  
5 model calibration. Zaghoul and Kiefa (2001) employed neural network procedures for the calibration of  
6 the Transport block in SWMM.

7 Real-time streamflow predictions. Several papers examined real-time predictions of stream flows. Chang  
8 and Chen (2001) employed neural network and fuzzy arithmetic tools to enhance predictions for the Da-  
9 cha River, in central Taiwan. They (Chen et al., 2001b) also used the method to predict floods one hour in  
10 the future for the Chingshui River during tropical storms using the new models. Khu et al. (2001) and  
11 Soon et al. (2001) employed genetic programming to improve real time forecasting for the Orgeval  
12 catchment in France. They successfully improved the forecasting for future periods up to the time of  
13 concentration of the watershed. Their comparisons showed that genetic programming was a better  
14 updating tool for real-time flow forecasting than previous methods. Xiong et al. (2001) combined the  
15 simulation results from different models to improve real-time forecasts.

16 Rainfall-runoff models and new programming tools. Whigham and Crapper (2001) provided the following  
17 definition: "Genetic programming is an inductive form of machine learning that evolves a computer  
18 program to perform a task defined by a set of presented (training) examples and has been successfully  
19 applied to problems that are complex, nonlinear and where the size, shape, and overall form of the  
20 solution are not explicitly known in advance." Genetic programming of rainfall-runoff model has also been  
21 used by Muttill and Liong (2001). Mathematical methods using linguistic variables (such as the application  
22 of fuzzy rule-based modeling), rather than conventional numerical variables, are starting to be used for  
23 hydrological studies (Hundecha et al., 2001). Ozelkan and Duckstein (2001) also recently applied fuzzy  
24 logic to rainfall-runoff modeling.

25 Water quality modeling and pollutant transport. Sanders et al. (2001) developed a hydraulic model to  
26 examine the tidal transport of stormwater bacteria near the southern California network of flood control  
27 channels that drain to near-shore bathing waters. Yuan and Oldham (2001) prepared a model describing  
28 the accumulation and washoff of urban area particulates, specifically examining metal binding. They  
29 monitored 47 rain events with short time intervals and obtained good agreement for predicting lead  
30 transport. Zoppou (2001) compared several different stormwater models, including statistical, empirical,  
31 hydraulic and hydrological model types.

32 Watershed model water balance. Jia et al. (2001) developed a distributed hydrological model, the Water  
33 and Energy Transfer Processes Model (WEP), to simulate spatially variable water and energy processes  
34 in watersheds having complex land covers. Variables include depression storage on land surfaces and  
35 canopies, soil moisture content, land surface temperature, groundwater tables, and water stages in rivers.  
36 Infiltration excess during heavy rains is simulated using a generalized Green-Ampt model, while flow  
37 routing is simulated using a one-dimensional kinematic wave method. The model was applied to the Ebi  
38 River watershed (27 km<sup>2</sup>) with a grid size of 50 m and a time step of 1 h. The model was verified through  
39 comparisons of simulated river discharges, groundwater levels, and land surface temperatures with the  
40 observed values. A comparison between the water balances at the present time and in the future was  
41 also conducted. It was found that the hydrological cycle in the future can be improved through the use of  
42 stormwater infiltration. Santini et al. (2001) modeled the Detroit Water & Sewerage Department collection  
43 system to identify flow sources. They also modeled the time needed to return to base flow conditions after  
44 wet weather events.

#### 45 *Model Applications*

46 Rainfall. NEXRAD (WSR-88D) radars have made spatially distributed rainfall data available in an  
47 operational environment (Carpenter et al., 2001). The present study addressed the use of NEXRAD data  
48 using a Monte-Carlo sensitivity analysis of event streamflow to parameter and radar input for the Illinois  
49 River basin in Oklahoma and Arkansas. The main conclusions was that the distributed model forced by  
50 NEXRAD data produces results comparable to those produced by the operational spatially-lumped  
51 models using raingage data. Detection of rain/no-rain condition on the ground is needed to apply radar  
52 rainfall algorithms to hydrological models (Lui et al., 2001d). A radial basis function (RBF) neural network-

1 based scheme for rain/no-rain determination on the ground using vertical profiles of radar data was  
2 described in this paper. Evaluation based on WSR-88D radar over central Florida indicates that rain/no-  
3 rain condition could be inferred fairly accurately. Lui et al. (2001e) presented a novel scheme for  
4 adaptively updating the structure and parameters of the neural network for rainfall estimation. The  
5 network can account for any variability in the relationship between radar measurements and precipitation  
6 estimation and also incorporate new information to the network without retraining the complete network  
7 from the beginning.. To make the monthly satellite data useful for hydrological applications (i.e. water  
8 balance studies, rainfall-runoff modeling, etc.), the monthly precipitation estimates must be disaggregated  
9 into shorter time intervals (Margulis and Entekhabi 2001). In this study, two simple statistical  
10 disaggregation schemes were developed for use with monthly precipitation estimates from satellites.  
11 Results suggested that one of the proposed disaggregation schemes could be used in hydrological  
12 applications without introducing significant error.

13 Rainfall data collected since the publication of NWS' Technical Paper 40 and HYDRO-35, and NOAA's  
14 Atlas 2, and the development of improved statistical methods, motivated several states to initiate update  
15 studies of precipitation distributions (Durrans and Brown 2001). Results of the Alabama study were  
16 disseminated via an Internet-based graphical user interface, which permits users to interactively point and  
17 click on a location of interest and have IDF curves or storm hyetographs returned on demand. The study  
18 by Kim and Barros (2001) modified the existing ANN model to include the evolving structure and  
19 frequency of intense weather systems in the mid-Atlantic region of the US for improved flood forecasting.  
20 Besides using radiosonde and rainfall data, the model also used the satellite-derived characteristics of  
21 storm systems such as tropical cyclones, mesoscale convective complex systems, and convective cloud  
22 clusters as input. The results from the application of the quantitative flood forecasting model in four  
23 watersheds on the leeward side of the Appalachian mountains in the mid-Atlantic region were presented.

24 Rainfall-runoff hydrological response. ANN methodology was applied to solve various problems  
25 concerned with hydrology and water resources engineering and planning, in particular the prediction of  
26 the index flood for several ungaged catchments in the UK (Dastorani and Wright 2001). A network with 7  
27 inputs provided the best results. The Multi-Layer Perceptron network with three layers, Tanh function for  
28 hidden layer, and the Sigmoid function for output layer were the most accurate. The review by Dawson  
29 and Wilby (2001) considered the application of ANNs to rainfall-runoff modeling and flood forecasting,  
30 including the related themes of the division and preprocessing of data for model calibration/validation;  
31 data standardization techniques; and methods of evaluating ANN model performance. A template was  
32 proposed to assist the construction of future ANN rainfall-runoff models. ANNs was shown to be an  
33 efficient way to model the runoff process where explicit knowledge of the internal hydrology is not  
34 required (Ahmad and Simonovic 2001). ANN was used for the Red River in Manitoba, Canada to predict  
35 the peak flow, timing and shape of runoff hydrograph, based on the antecedent precipitation index, melt  
36 index, winter precipitation, spring precipitation, and timing. The percent error in simulated and observed  
37 peak flow and time of peak was 6 and 3.6 %, respectively.

38 A rainfall-runoff model was developed based on water balance equations (Abulohom et al., 2001). The  
39 model inputs were precipitation and potential evapotranspiration (both on monthly basis) and the output  
40 was the simulated runoff at the watershed outlet. The model was calibrated and tested for four  
41 watersheds, with a correlation coefficient between the simulated and measured data ranging between  
42 77% and 93%. Croke and Jakeman (2001) assessed the model types available to improve the prediction  
43 of catchment flows and transport in Australia where the flows are typically peakier, base flows are of lower  
44 proportion, runoff coefficients are smaller, and dry periods are longer and more variable, than in  
45 European and North American catchments. Improvement of predictions relied on the following: more  
46 rigorous testing of models; the use of improved interpretation of spatial data; more and better monitoring  
47 of hydrological response; complementary use of conceptual and distributed models; and integration of  
48 modeling with other information.

49 The Garg and Sen (2001) study presented a physically based hydrologic model using derived watershed  
50 features to simulate rainfall-runoff response of a catchment. The finite-element concept was used to  
51 obtain the time-invariant weighting coefficients for estimating the rainfall on the cascade planes. Overland  
52 flow was simulated using a kinematic wave model. The fundamental premise of the AFDC approach is  
53 that maintenance of a stream's ecological integrity depends upon maintaining an appropriate flow regime  
54 (magnitude and frequency) (Good and Jacobs 2001). The AFDC provided a graphical tool to illustrate the

1 quantity and frequency of streamflow available in a river basin and facilitated the simulation of the  
2 modified streamflow regime based on historic time series data. The AFDC methodology was illustrated in  
3 the lower Suwannee River basin in Florida. The Long-Term Hydrologic Impact Assessment (LTHIA)  
4 model run on a GIS is a relatively simple model that uses the Curve Number method to estimate changes  
5 in surface runoff between different stages of development (Grove et al., 2001). Application to a large,  
6 rapidly urbanizing watershed near Indianapolis, Indiana suggested that average annual runoff depths  
7 increased by more than 60% from 1973 to 1991. A sensitivity analysis showed that a precipitation record  
8 length of 15 years or more was required to produce consistent results with LTHIA.

9 Chen and Cai (2001) used the Kinematic Wave method to model the rainfall-runoff process in an  
10 idealized drainage basin. The simulation results showed that for basins with moderate to high rates of  
11 infiltration losses, the critical rainfall duration associated with the maximum peak discharge may be  
12 shorter than the time of concentration ( $t_c$ ) for the basin, implying that a storm with a spatial coverage of  
13 only part of the basin may generate the maximum peak discharge. For basins with low or negligible  
14 infiltration losses, the critical rainfall duration approaches  $t_c$ . The geomorphological instantaneous unit  
15 hydrograph (GIUH) was used to relate the shape and scale of the catchment transfer function to stream  
16 network topology and channel characteristics (Hall et al., 2001c). GIUH derivation depended on a series  
17 of assumptions, including the estimation of a "characteristic velocity." If this velocity is expressed in terms  
18 of the kinematic wave approximation, the peak and time-to-peak of the IUH may be expressed by a group  
19 of catchment and channel characteristics and by the intensity of rainfall excess. The study by Guo  
20 (2001a) expanded the rational method into the rational hydrograph method in which the time of  
21 concentration is considered the system memory and the contributing rainfall depth to the present runoff  
22 rate is defined as the accumulated precipitation over the past up to the time of concentration. Using this  
23 method, the complete runoff hydrograph could be generated under a continuous nonuniform hyetograph.  
24 A new formula for estimating the time of concentration was derived from 25 rainfall/runoff events in four  
25 urban watersheds and was confirmed in watersheds in Maryland and Colorado.

26 Neither the index antecedent precipitation index and the Natural Resource Conservation Service's  
27 antecedent moisture condition triad consistently characterized the runoff consequence of watershed  
28 moisture preceding a rainfall event (Heggen 2001). A normalized antecedent precipitation index that  
29 modified the conventional index in three ways (inclusion of antecedent precipitation earlier in the day of  
30 the event, normalization to the station mean, and normalization to the antecedent series length) was  
31 proposed. Initial results showed the proposed index outperformed single curve number-based results,  
32 even when the curve number is fit to historic rainfall-runoff records.

33 Kroll et al. (2001) tested whether the inclusion of new watershed characteristics improved the prediction  
34 of extreme hydrologic events, in particular, low streamflow prediction. Preliminary results indicated that  
35 regional regression models of low streamflow quantiles, which traditionally have very high model errors,  
36 might be improved in some regions by including topographic, climatic, and hydrogeologic statistics. Lee et  
37 al. (2001a) developed a stochastic differential equation (SDE) for a lumped rainfall-runoff model and  
38 applied it to a watershed that consisted of a number of subwatersheds in series in Taiwan. The  
39 development of the moment equations of simulated outflow was based on a SDE. The outflow  
40 hydrograph was obtained by applying the Laplace transform method to the equations that describe rainfall  
41 excess. A second paper by Lee et al. (2001b) presented a geomorphic and hydrologic information system  
42 for calculating the discharge in small and midsize ungaged watersheds. A compound module, based on a  
43 digital elevation model, frequency analysis theory, kinematic-wave approximations, and geomorphic-  
44 based runoff modeling, provided enough information for calculating the design discharge for different  
45 return periods. The stochastic-conceptual rainfall-runoff simulator (SCRRS) developed by Freeze in 1980  
46 was used to demonstrate quantitatively the interplay of the factors that control overland flow by the Horton  
47 and Dunne mechanisms (Loague and Abrams 2001). The simulation domain and input data were  
48 abstracted from the R-5 catchment (Chickasha, OK) data sets. The SCRRS simulations showed that the  
49 Horton and Dunne processes can (i) occur simultaneously at different locations during a given rainfall  
50 event, (ii) change from one process to the other with time depending on the characteristics of the rainfall  
51 event, and (iii) be strongly dependent on the initial conditions.

52 Corrêa et al. (2001) presented the outcome of recent work for the cities of Springfield and Columbia,  
53 Missouri using high spatial resolution satellite data to enhance or effectively replace conventional data  
54 sources for mapping impervious and bare ground surfaces, and for determining runoff curve numbers

1 (CNs) required by hydrologic and environmental computer models. The estimated accuracy of high-  
2 spatial resolution imagery was approximately 80%. Spatial data of primary importance to hydrologic  
3 modeling include Digital Elevation Models (DEM) for topography, imagery such as Digital Raster Graphics  
4 (DRG), hydrologic soil type, and land use for infiltration losses (Hartman and Nelson 2001). Other  
5 supporting data types may include Triangulated Irregular Networks (TIN), hydrography, precipitation, and  
6 stream stage. The GSDA website offered detailed explanations, tips, and direct links to hydrologic data  
7 sources. A case study modeling the Lost Creek watershed in Northeastern Utah provided an overview of  
8 the data acquisition process, and can also be found at the GSDA website. Land use changes over a 30  
9 year period (1961-1990) were incorporated into a continuous simulation rainfall runoff model to  
10 investigate the effect of these changes on flood frequency in the Thames catchment at Kingston (Crooks  
11 and Davies 2001). Broad scale changes in land use over the last 120 years in the Thames catchment  
12 were determined from a variety of sources.

13 PCSWMM was used to calibrate the kernel function in the central rain-runoff algorithm in SWMM and to  
14 generate plots that were compared to the experimental results obtained from a laboratory rig whose  
15 impervious pavement had a 0.025 m/m/ slope over an area of 2.11 m<sup>2</sup> (James 2001). Results indicated  
16 that the RUNOFF algorithm produces reasonable results, even when supercritical laminar flow cases  
17 were included in the validation tests. Andreassian et al. (2001) proposed a new approach to sensitivity  
18 analysis for watershed models through a comparison between the efficiency ratings and parameter values  
19 of the models and the quality of rainfall input estimate (GORE and BALANCE indices). Although the  
20 watershed size was generally immaterial, smaller watersheds appeared to need more precise areal  
21 rainfall estimates to ensure good modeling results.

22 Rainfall-runoff floods. Engineering practice and works design requires the calculation of the “design flood”  
23 (Paoli et al., 2001). When there are no available records for the flood frequency analysis, different  
24 formulations, semi-empirical methods and hydrological simulation models have been used with the design  
25 storm then transformed into a maximum flow. This research concluded that the procedure to determine  
26 the design flood from frequency analysis of flow peaks (obtained of the transformation P-Q) provided  
27 better results than the one that transformed the storm design. Three indirect techniques for index flood  
28 estimation were analyzed in order to evaluate their applicability and effectiveness (Brath et al., 2001). The  
29 results showed that the statistical model had a better descriptive ability than the physically-based models.  
30 The results highlighted that direct estimation techniques were advisable for catchments with peculiar  
31 geomorphoclimatic properties.

32 One-dimensional floodplain models of the proposed modifications to William Cannon Drive in Austin,  
33 Texas, indicated no significant upstream impact during the 100-year flood (Buechter 2001). Given the  
34 limitations of traditional one-dimensional tools in this application TxDOT decided that to model this project  
35 using two-dimensional floodplain modeling techniques. The two-dimensional model better identified the  
36 potential impacts associated with the proposed highway reconstruction. Radar-rainfall data, remotely-  
37 sensed land-use and land-cover data, measured streamflows, and meteorological data were incorporated  
38 into the distributed flood forecasting model WATFLOOD to synthesize runoff hydrographs for three  
39 significant rainfall events that occurred in 1995 in the Duffins Creek drainage basin in southern Ontario  
40 (Cranmer et al., 2001). These results indicated that WATFLOOD could accurately model the nonlinear  
41 rainfall-runoff processes for increasing rainfall intensities with respect to peak flow, basin lag, and time to  
42 peak flow. Iturbe et al. (2001) used the spatial structure of storms to enhance flood estimation using a  
43 rainfall runoff process. The results suggested that interpolation was insensitive to the variogram selection.  
44 The applications of the estimation error (and of the method in general) were discussed.

45 CSO models. Modeling tools such as SWMM typically have been used to model I/I with sanitary collection  
46 systems (Czachorski and Van Pelt 2001). The hydrograph theory used by these models explicitly  
47 assumes that the system can be accurately modeled by linear dynamics; however, in general, this  
48 assumption is inaccurate. Nonlinear dynamics result from complex flow patterns and other transport  
49 mechanisms, but typically the dominant nonlinearity can be attributed to antecedent moisture effects.  
50 SWMM was used by Onondaga County, New York to simulate the hydrologic and hydraulic elements of  
51 its combined sewer system and to project the effects of a range of abatement alternatives (Davis et al.,  
52 2001c). The models successfully identified the maximum conveyance capacity of the collection system  
53 and showed that, following abatement, 85% of the combined sewage would be captured for primary  
54 treatment at the WWTP. To assist with hydraulic analysis for a Portland, Oregon project to reduce the

1 potential for basement flooding, a detailed SWMM model of the basin using a suite of GIS and database  
2 tools was developed. The detailed GIS approach with virtual raingages produced model results that  
3 correlated very well with flow monitoring data. The detailed SWMM model identified the location of much  
4 of the system capacity problems. The City of Winnipeg initiated a basement flood relief program in the  
5 mid-1960s to provide a minimum five-year level of protection to the residents (Steiss and Watters 2001).  
6 Since 1977, this program has been modeled with SWMM, and to date, sewer relief works have been  
7 implemented in 21 of the City's combined sewer districts. The public notification method developed for the  
8 City of Detroit uses a continuous SWMM model for determining the occurrence and time of overflow and  
9 was approved as an interim reporting method until an instrumentation project currently under way is  
10 completed (Brink et al., 2001).

11 James and Young (2001b and c) developed new code for radial gates that was integrated into the  
12 Extended Transport (EXTRAN) module of SWMM. The new program, called RGEXTRAN, was capable of  
13 handling unsteady flow conditions occurring in real sewer systems with radial and regular sluice gates.  
14 The program also provided for dynamic control of the radial gates (or other gate types) based on flow  
15 conditions at remote locations in the sewer system. James and Young (2001a) demonstrated that  
16 RGEXTRAN could be used to optimize a combined sewer system operation by fully utilizing all available  
17 storage and conveyance capacity in the existing facilities. The program was applied to the combined  
18 sewer system in Vancouver, British Columbia, and verification tests demonstrated that the new program  
19 accurately represents the operation and hydraulics of the dynamic radial gates and siphon-weirs found in  
20 the system. Due to the complex number of available pathways that RDII may enter a sanitary sewer,  
21 rainfall dependent infiltration and inflow (RDII) has been one of the most difficult components of the urban  
22 wet-weather water budget to estimate (Wright et al., 2001). Various tools have been applied to estimating  
23 this special hydrologic response, including the rational method and several unit-estimation methods. The  
24 paper highlighted results from a relatively new physically based conceptual method first introduced by  
25 Kadota and Djebbar in 1998, a modification to the SWMM RUNOFF module that includes the response of  
26 a conceptual non-linear reservoir to changes in groundwater elevations resulting from permeable-area  
27 infiltration.

28 MOUSE, an Arcview-GIS based dynamic model, was selected to analyze the performance of the  
29 combined sewer overflow (CSO) system operated by the City of New Haven (Connecticut) and the New  
30 Haven Water Pollution Control Authority for 3-month, 6-month, 1-year and 2-year design storms (Cheung  
31 et al., 2001). In addition, an average rainfall year was simulated to determine the performance of the  
32 system under typical rainfall conditions. The short-term control plan identified minor adjustments to  
33 overflow weirs and orifices, and to pump rates, while a long-term control plan included major capital works  
34 such as storage tanks or pump station and trunk sewer replacement. Long term numerical modeling was  
35 used to verify design parameters for the new King County 7-million-gallon CSO storage and conveyance  
36 tunnel, including gate- and pump-operation rule sets for operating the storage systems (Crawford and  
37 Swarner 2001). Additional modeling was performed to determine operation strategies for extreme events  
38 such as combinations of large storms, high tides, or unusual flow diversions. The long term numerical  
39 modeling verified the suitability of rule sets for conditions which often cannot be anticipated.

40 Harwood and Saul (2001) reviewed some of the physical model studies which have been carried out in  
41 the UK to determine the performance of combined-sewer overflow chambers. The limitations of both  
42 physical modeling and computational fluid dynamics approaches were discussed, and it was concluded  
43 that the future of chamber modeling combine both approaches. The impacts on DO in the Rouge River  
44 from CSO-treatment basins were evaluated using basin effluent monitoring, river monitoring and dynamic  
45 water quality modeling (Kluitenberg et al., 2001). Using a consensus-based approach, work groups  
46 including State personnel, CSO community representatives and consultants concluded that the  
47 demonstration basins could eliminate raw sewage, protect public health and achieve water quality  
48 standards, including the DO standard.

49 Reeves and Lewy (2001) discussed the representation of long term, highly attenuated runoff in collection  
50 systems, focusing on rainfall dependent groundwater infiltration. Contributions to urban drainage systems  
51 by stream ingress or from large permeable areas do not fall easily into either the runoff or baseflow  
52 categories. In addition, the increasing prevalence of Sustainable Urban Development (SUD) structures is  
53 promoting a need to represent highly attenuated groundwater contribution. The paper by Miralles et al.  
54 (2001) described the methodology that was used to evaluate wet weather flows within the sanitary sewer

1 system in Luquillo, Puerto Rico. The methodology, based on a unit hydrograph model to predict (RDI/I),  
2 was calibrated by comparing predicted wet weather flows to those observed in the five flowmeter  
3 locations.

4 For municipalities to cost-effectively plan, organize, and implement infrastructure improvements, they  
5 require improved information on structural conditions, decision-making tools, operation and maintenance  
6 practices, and techniques for repair and rehabilitation (Tafari et al., 2001). The paper reviewed European  
7 approaches for diagnosing and analyzing water and wastewater systems, and in particular the use of  
8 performance indicators and non-hydraulic models for predicting failures in these systems.

9 Pollutant sources and transport. Burian et al. (2001b) investigated the suitability of integrating  
10 deterministic models to estimate the relative contributions of atmospheric dry and wet deposition by  
11 stormwater runoff. The CIT airshed model and the SWMM were linked to simulate the fate and transport  
12 of nitrogen species through the atmosphere and storm drainage system in Los Angeles, California, USA.  
13 Coupling CIT and SWMM involved defining and resolving five critical issues: (1) reconciling the different  
14 modeling domain sizes, (2) accounting for dry deposition due to plant uptake, (3) estimating the fraction of  
15 deposited contaminant available for washoff, (4) defining wet deposition inputs to SWMM, and (5)  
16 parameterizing the SWMM washoff algorithm.

17 A mass-balance model for calculation of annual metal loads to lakes dominated by diffuse inflow and  
18 urban runoff was presented by Lindstrom and Hakanson (2001). In the lake, the variations in runoff  
19 concentrations were smoothed, making it possible to get a more stable flow assessment. The model  
20 accounted for inflow, outflow and sediment interactions (sediment resuspension is modeled using new  
21 approaches) with the most important factors in the uncertainty of model predictions being the settling  
22 velocity of particulate matter, the percentage of accumulation areas for fine particles, and the dissolved  
23 fraction of the total metal concentration. High correlations between metal accumulation and phosphorous  
24 and nitrogen concentrations indicated that the autochthonous lake production was crucial. The sources,  
25 distribution and fate of fecal coliform populations in the North Fork of the New River in Fort Lauderdale,  
26 Florida were investigated by Scarlatos (2001). In order to facilitate field sampling, computer simulations  
27 using WASP were applied to assess the likelihood of the various possible pollution scenarios. WASP  
28 accurately simulated the water hydrodynamics and coliform concentrations within the North Fork, while  
29 the neural network identified correlations between fecal coliform and the other parameters.

30 Swanson and Ward (2001) presented a series of applications of WQMAP, an integrated model of the  
31 circulation and pollutant transport in estuarine and coastal waters, to Narragansett Bay. Applications  
32 included circulation and temperature distributions in Mt. Hope Bay; consideration of the effects of a once-  
33 through cooling water discharge from a fossil fueled power plant; circulation in the Providence River and  
34 Upper Narragansett Bay; fecal coliform transport from combined sewer overflow (CSO) loads; and a  
35 eutrophication study of the Providence River.

36 BMP performance. Stormwater management systems for new urban development were traditionally  
37 analyzed using computer models employing design storm events, rather than continuous modeling using  
38 long term historical rainfall data and associated frequency analyses (Farrell et al., 2001). The differences  
39 in the results (i.e., designs) generated by the two methods have not typically been understood during the  
40 planning and design process. The different flow and storage regimes in the Town of Milton (Sixteen Mile  
41 Creek Watershed) generated by the alternate methods have been highlighted, along with a number of  
42 modeling and physical factors which are considered to contribute to the differing results. The Clinton  
43 River Watershed Council selected the Source Loading and Management Model (SLAMM) as the main  
44 instrument in its watershed plan development (Myllyoja et al., 2001). SLAMM accurately computed runoff  
45 pollutant loads and flows associated with small storm events, which was critical because most of the  
46 pollutant load was associated with the smaller, frequent runoff events. SLAMM evaluated several control  
47 practices including detention ponds, infiltration devices, porous pavements, grass swales, catchbasin  
48 cleaning, and street cleaning. A numerical model (sediment trap efficiency for small ponds-STEP) was  
49 developed to simulate sediment deposition in small ponds (less than or equal 1 ha) and to calculate the  
50 sediment trap efficiency (STE) (Verstraeten and Poesen 2001). Eight runs with an experimental pond in  
51 Belgium were used to test the model. The STEP model produced reasonable predictions of STE as well  
52 as the shape and magnitude of the effluent sediment concentration graph.

1 Huber (2001) summarized the ability of SWMM to simulate wet-weather controls favored in current  
2 practice, including those related to LID. The model simulated some practices well, such as storage, and  
3 other options not as well, such as wetlands and filtration. In designing a permeable pavement installation,  
4 surface infiltration capacity that allows an adequate volume of stormwater to be captured and treated by  
5 the facility must be provided and maintained (James et al., 2001). The paper detailed the underlying  
6 method and function of a free-ware program that uses SWMM for the design of permeable pavement  
7 installations.

8 The water balance model (Aquacycle) developed in this study represents water flows through the urban  
9 water supply, stormwater, and wastewater systems (Mitchell et al., 2001b). The daily time step provides  
10 temporal flow distribution and enables comparison of the different components of urban water demand.  
11 Aquacycle was tested using data from the Woden Valley urban catchment in Canberra, Australia and  
12 could satisfactorily replicate its water supply, stormwater and wastewater flows.

### 13 *Watershed Management and TMDLs*

14 EPA's Technical Support Document for Toxics (1991) provided recommendations on evaluating  
15 frequency, magnitude, and duration issues, including the use of statistical permit limit derivations and  
16 techniques for dynamic wasteload analysis (Butcher and Diamond 2001). A survey of state permitting  
17 agencies found that few of these recommendations have been put into practice. Instead, most  
18 jurisdictions continued to rely on steady-state wasteload analyses. The WERF research project (98-HHE-  
19 3) is developing an Implementation Guidance document (with software) to aid permittees in deciding  
20 whether to undertake these dynamic analyses and to aid in implementing the analyses under the existing  
21 guidance. Information technologies and watershed management approaches have enabled decision-  
22 makers to establish predictive relationships between different land-use activities and the sediment and  
23 nutrient loads discharged from the watershed (Khairy et al., 2001). These approaches then have been  
24 extended to assess the impacts of these wasteloads on the receiving aquatic systems. A case study on  
25 the simulation of Tangipahoa Watershed and Lake Ponchartrain Ecosystems was used. The Integrated  
26 Plan for the Wastewater Program (IPWP) was a public-driven effort designed to develop policies for the  
27 planning and implementation for future wastewater collection, treatment, reuse and disposal projects  
28 (Lopez-Calva 2001). The integrated systems model was developed to measure the performance of policy  
29 alternatives in relation to a set of objectives and performance measures established by stakeholders. The  
30 conceptual character of the model proved to be appropriate for this policy-planning project. Monte Carlo  
31 simulations, which account for uncertainty in model parameters, were performed for a model of a 10-km  
32 stretch of the River Cam in Cambridge in Eastern England (Duchesne et al., 2001). The simulation results  
33 were used to rank wastewater treatment plant control strategies according to their impacts on river water  
34 quality. It was found that ranking was robust even with the uncertainty in the parameter values for the  
35 control strategies.

36 The BASINS system was developed to provide technical tools to support EPA's regulatory water quality  
37 programs, in particular the TMDL program (Cocca 2001). BASINS 3.0 provides a range of detail in  
38 watershed models and provides databases, data management tools, documentation, and other resources  
39 to make modeling waterbodies and watersheds easier. The three watershed models in BASINS 3.0  
40 include the complex Hydrological Simulation Program Fortran (HSPF) model, the complex Soil and Water  
41 Assessment Tool (SWAT) model; and the simple PLOAD model. Each model has an automated GIS data  
42 extraction script. The paper by Dorn et al. (2001) reviewed BASINS-STAR (BASINS STrategy, Analysis,  
43 and Reporting system), a set of tools to assist decisions-makers identify alternative management  
44 strategies. The main engine of BASINS-STAR is a genetic algorithm-based optimization technique.  
45 BASINS-STAR forms a decision support framework for watershed management (Murray et al., 2001b).  
46 The capabilities of this framework were demonstrated through several scenarios for the Suwanee Creek  
47 watershed in Georgia. The paper illustrated a means by which a decision-maker can determine maximum  
48 allowable level of land development in a given watershed while maintaining a target level of water quality.  
49 The effect of BMPs on land use planning was also explored.

50 The main concern of using continuous simulation approach is that the use of data during specific  
51 representative hydrologic period does not necessarily cover the most "critical" condition and it is very data  
52 intensive (Zhang et al., 2001). A practical, event-based Critical-Flow Storm (CFS) was developed with  
53 application in the Nitrate TMDL Development for Muddy Creek/Dry River in Virginia using BASINS/HSPF.

1 As an alternative method for TMDL development, the CFS approach gives reasonable results and  
2 explicitly addresses the critical condition as a combination of stream flow, magnitude of storm event, and  
3 initial watershed condition. The Northeast Georgia Regional Development Center has taken a proactive  
4 stance in protecting the Alcovy River watershed (Jha et al., 2001). This project discusses the modeling  
5 portion of these larger studies. The purpose of this study was to develop a calibrated watershed water  
6 quality model in order to simulate the impact of different future scenarios. The BASINS interface to HSPF  
7 was selected as the watershed water quality model for the Alcovy River watershed. Water, sediment,  
8 phosphorus and fecal coliform were the constituents modeled. Eight future scenarios incorporating  
9 various management practices were modeled. These scenarios include the simulation of conservation  
10 subdivisions, riparian buffer ordinances, impervious surface restrictions, storm water quality controls and  
11 improved enforcement of erosion and sedimentation controls. Modeled results indicate that a combination  
12 of BMPs must be implemented in order to reduce pollutant loading to the Alcovy River.

13 Rockdale County, Georgia assessed its watershed in order to develop a management plan that combines  
14 water quality protection with other objectives, including expansion of water supply and wastewater  
15 treatment capacity (Clements et al., 2001). An innovative planning process linked management  
16 objectives, assessment information, predictive modeling tools, and stakeholder participation to risks. The  
17 results were effective for evaluating management options. The paper by Dymond et al. (2001) described  
18 the web-enabled integrated modeling system, including data sources, collection, analysis methods,  
19 system software and design, and issues of integrating the various component models. The integrated  
20 system contained three modeling components, namely hydrology, economics, and fish health. Earles and  
21 Jones (2001b) presented a water quality model developed for evaluating pollutant loading from  
22 development projects in the Aurora Reservoir watershed in the Denver metropolitan area, Colorado.  
23 Laing/Village Homes developed the model to evaluate pollutant loadings from the development; Aurora  
24 determined the allowable pollutant loadings to the reservoir. A linked watershed and water quality model  
25 was applied to the Little River watershed, Georgia (Moskus et al., 2001). The model was a modified  
26 version of the Generalized Watershed Loading Function (GWLF) model that had been linked to a  
27 simplified transport model based on Water Quality Analysis Simulation Program (WASP). This model was  
28 to forecast water quality under future development conditions for flow, sediment, phosphorus, bacteria,  
29 and metals. Future scenario runs showed that water quality standards likely will be violated in the future.  
30 The WERF project 97-IRM-5E modeled the trading market in Maryland under various assumptions with  
31 the results used to design and implement a statewide trading program (Bacon et al., 2001). Credits could  
32 be created and sold by POTWs, as well as nonpoint sources such as urban, agricultural, and  
33 undeveloped land. This paper presents preliminary results from the first round of several trading  
34 scenarios. Water quality modeling was conducted to examine the effects of growth in the McDowell Creek  
35 Basin on the water quality in Mountain Island Lake (Quinlan et al., 2001). The water quality modeling  
36 included both a watershed and a lake model. This study investigated the effects of effluent discharges  
37 ranging from the existing 6 mgd up to 24 mgd due to basin development. Nonpoint source loadings were  
38 estimated at each level of development and were included. Results of the modeling showed that as the  
39 treatment plant is expanded, a reduction in effluent nutrient concentrations will eventually be required to  
40 maintain acceptable water quality.

#### 41 *Geographic Information Systems (GIS)*

42 The residual inflow to combined sewers must be quantified it affects the CSO activation frequency and  
43 thus the degree of roof drainage separation that must be undertaken to eliminate CSOs (Ho et al.,  
44 2001a). Hydrologic models have employed coefficients to represent the directly connected impervious  
45 area (DCIA), with the coefficients being proportional to the total amount of impervious area in the region.  
46 Better estimation of the DCIA requires sufficient geographic information and field data. A GIS application  
47 was developed to automate the process of calculating drainage utility apportionment percentages  
48 (Hughes et al., 2001). The application used the rational formula to estimate stormwater runoff. The  
49 application was applied to the Twelve Towns Drainage District (Oakland County, Michigan) 2000  
50 Apportionment for Improvements to their combined sewer overflow retention treatment facility.

51 The application of HEC-HMS, HEC-RAS and HEC-GeoRas, coupled with GIS and NEXRAD radar,  
52 efficiently analyzed proposed alternatives to previous Army Corps of Engineers' plans including voluntary  
53 floodplain property buyouts and various, smaller-scale channelization schemes in Clear Creek  
54 (Benavides et al., 2001). Applying the latest HEC tools, NEXRAD and GIS to test the viability and

1 effectiveness of specific flood control alternatives provided acceptable results. An ArcView GIS interface  
2 was created to view and facilitate the development of EPA's SWMM RUNOFF and EXTRAN models  
3 (Heineman 2001). The interface contained Avenue scripts that allowed the user to visualize a SWMM  
4 model in conjunction with existing GIS data. The scripts permitted viewing of model input and output  
5 summary data within ArcView, but could not substitute for existing SWMM-commercial software  
6 interfaces, since they do not permit viewing of conduit profiles, dynamic display of results, or editing of  
7 input data. The EPA SWMM model was a major tool of the stormwater management master plan for  
8 Hillsborough County, Florida (Ho et al., 2001b). Hillsborough County coupled the SWMM model with a  
9 county-wide GIS model. Hussein (2001) reviewed the MODFLOW Hydrologic Modeling System  
10 (MODFLOW-HMS), a physically based model capable of simulating the land phase of the hydrologic  
11 cycle including overland and channel flow on the surface, variably saturated flow in the subsurface, as  
12 well as transport during flow in both systems. GIS utilities were developed to estimate model parameters.  
13 The paper discussed the algorithms used to process these data and estimate the spatial and temporal  
14 variation in parameters over the finite-difference mesh, using a Mad River, Ohio study as a case study.  
15 Johnson County, Kansas embarked on an aggressive program to complete stormwater master plans and  
16 new floodplain delineations county-wide (Koch et al., 2001). The primary goals included (1) developing  
17 new hydrologic and hydraulic models for both existing and ultimate development conditions; (2) updating  
18 the existing FEMA floodplain boundaries for existing development conditions; (3) providing local  
19 communities with a floodplain management tool; (4) defining flood-prone areas; (5) developing sound  
20 engineering and bio-engineering solutions to alleviate identified problem areas; and (6) developing GIS-  
21 based stormwater data for ongoing asset management purposes. HEC-1 was used for hydrologic  
22 analyses, and HEC-RAS for hydraulic analyses. GIS techniques were used to automate the process of  
23 model development.

24 Two of the most data-intensive programs in use include computerized maintenance management  
25 systems (CMMS) and GIS (Ratliff and Schmitz 2001). The integration of these systems would result in  
26 greater efficiency of resources, improved data collection processes, and reduced data redundancy and  
27 data maintenance requirements. The paper discussed the major phases of a project that included asset  
28 inventory and a multi-year condition assessment program for sanitary sewers and storm drains. This  
29 paper presented lessons learned from the project and highlighted recommendations for the successful  
30 integration of inventory and condition data into CMMS and GIS. Xu et al. (2001) reported on the  
31 integration of a physically-based distributed model with a GIS in watershed-based water resources  
32 management. The integration facilitated the examination of a wider range of alternatives than could be  
33 done using conventional methods, and provided a 'living' management tool that could be modified and  
34 updated if the watershed condition changed. With the increasing availability of digital and remotely  
35 sensed data such as land use, soil texture, and digital elevation models (DEMs), GIS has become  
36 indispensable in preprocessing data sets for watershed hydrologic modeling (Yu et al., 2001c). The  
37 transfer of inputs and outputs between the model and GIS can be greatly simplified by incorporating the  
38 model itself into the GIS environment. The authors incorporated a simple hydrologic model that used the  
39 curve number method of rainfall-runoff partitioning, a groundwater baseflow routine, and the Muskingum  
40 flow routing procedure, into the GIS model. Then the model was used to simulate the hydrologic response  
41 of the Upper West Branch of the Susquehanna to two different storms. The simulated hydrographs  
42 compared well with the observed hydrographs at the basin outlet.

43 Chester County, Pennsylvania municipalities have been working with Green Valleys Association to  
44 develop a "Sustainable Watershed Management Program" (Cahill et al., 2001). The heart of this program  
45 is a "Water Balance Model" that is interactively linked to a detailed GIS model. Municipalities can  
46 determine if the sustainable watershed goals are attainable or if changes are needed in municipal  
47 regulations. The visual component of the GIS allows the planners and municipal officials to quickly "see"  
48 the areas of concern, as well as the effects of regulatory changes. A comprehensive watershed approach  
49 to sewer separation planning was developed by the City of Atlanta, Georgia (Smith et al., 2001b). The  
50 holistic approach considered future land use and urban development within the combined sewer area  
51 basins, and determined the additional sewer conveyance capacity needed to accommodate the City's  
52 future growth. The GIS-based tool will provide Atlanta with the ability to store and analyze data related to  
53 existing and proposed land uses, transportation improvements, storm water detention and other water  
54 quality enhancement projects. Due to the pending SSO Rule and CMOM requirements, the City of  
55 Phoenix (COP) needed to manage the large amount of data generated by the required studies (Malone et

1 al., 2001). The COP used the Water Department's GIS database as the format. Direct electronic  
2 collection of data and a "hands-off" data transfer and collection of data into the COP's GIS was  
3 established.

4 A GIS model was developed for the Bushkill Creek in eastern Pennsylvania watershed to address the  
5 needs of various constituencies (Ruggles et al., 2001). This paper described the development of the GIS  
6 that supports the needs of each group having an interest in the Bushkill Creek watershed and provided  
7 examples of the information currently available. The application of GIS technology to the field of urban  
8 storm-water modeling was reviewed by Sample et al. (2001b), and an application in urban stormwater  
9 management at a neighborhood scale was presented. Using economic analysis to compare the cost of  
10 controls, including the total cost for land intensive controls, the optimal mix of BMPs was found. The  
11 paper by Shamsi and Cigana (2001) demonstrated the reduction of wet weather overflows by conducting  
12 TV inspection of sewers using multimedia computer technology and GIS, and using the information to  
13 improve the maintenance of sewer systems. The paper proposed a four-step method for implementing a  
14 GIS-based TV inspection program.

15 Shamsi (2001) showed how digital elevation models (DEM) can be used to develop hydrologic models  
16 using the off-the-shelf GIS software. The chapter showed how to automatically delineate watershed  
17 subbasins and streams using DEM. A case study was presented that compared the DEM results with the  
18 conventional manual delineation approach. The work described by Sibiga et al. (2001) attempted to  
19 quantify pollutant loading sources as a function of land use within the Cazenovia Creek sub-basin of the  
20 Upper Buffalo River watershed by applying a GIS-based Watershed Loading Model (WLM). The model  
21 was also used to evaluate reduction options for meeting water quality criteria in the Cazenovia Creek. A  
22 program developed within Arc-Info GIS was used to identify storm characteristics from a set of rainfall  
23 gages (Tsanis and Gad 2001). Three different interpolation techniques (spline, inverse distance weighted,  
24 and kriging) were used to visualize the spatial distribution of rainfall using an example in the Hamilton-  
25 Wentworth Region in Ontario, Canada. Supported by GIS, snowmelt runoff simulation models have been  
26 built for the Qushenan basin in the west of China (Wang and Li 2001). Digital Terrain Factors (DTF) were  
27 employed to divide the basin. The results showed that this approach was significant and practical.

## 29 **REGULATORY POLICIES AND FINANCIAL ASPECTS**

### 30 *Policy*

31 After over 20 years of progress, many rivers, lakes and stream still failed to meet water quality standards  
32 (Kwan 2001). Studies and monitoring data have shown that stormwater runoff was a major source of  
33 water quality impairment. Currently, five regional states have reported stormwater runoff as a major cause  
34 of water quality impairment. An effective, integrated and coordinated storm water enforcement strategy  
35 will need to be established in full partnership with the eight Region 4 states.

36 EPA has been completing a series of federal initiatives to address wet weather discharges due to CSOs  
37 and SSOs (Hall et al., 2001a). As a result, communities nationwide will need sewer system/treatment  
38 plant improvements to address these wet weather discharge issues. Little attention has been paid thus far  
39 to a series of subtle changes in EPA's implementation of its existing bypass regulations and secondary  
40 treatment requirements for controllable CSO and SSO flows. Reducing wet weather discharges is a core  
41 activity of EPA's Office of Wastewater Management (Cook 2001). The office's programs include  
42 combined sewer overflows, sanitary sewer overflows, storm water, and animal feeding operations.  
43 Several current regulatory initiatives have been designed to improve water quality by reducing the  
44 environmental impacts of wet weather events. Bell and Powell (2001) described the pending SSO  
45 regulations, where there are three standard permit conditions for sanitary sewer collection system  
46 owners: capacity, management operation and maintenance (CMOM); prohibition on SSOs; and reporting,  
47 record keeping and public notification. The paper focused on the CMOM aspect of the permit conditions.  
48 Although the SSO Rule is not currently effective, Sowatzka et al. (2001) advocated using the draft Rule to  
49 provide system owners and operators guidance on EPA's expectations for systems, including satellite  
50 facilities, for their (CMOM) program.

51 Centilla and Slack (2001) reported on the outreach efforts of the EPA. EPA has been working with  
52 stakeholders during the "toolbox" development. The "toolbox" includes fact sheets, case studies, technical

1 guidance documents, training and outreach efforts, sample self-audit reports, sample model ordinances,  
2 technical research, and compliance monitoring and assistance tools. The paper by Foess and McNitt  
3 (2001) presented the methodology, scope, and key results of collection system management, operation,  
4 and maintenance (MOM) program audits for two Florida wastewater utilities. Selected MOM program  
5 performance metrics for the two utilities were compared to data from other well-operating utilities and  
6 published guidelines in order to assess the effectiveness of the programs. In mid-2000, the City of  
7 Thousand Oaks's Public Works Department updated its Wastewater System Master Plan (Giguere et al.,  
8 2001). Anticipating the future Sanitary Sewer Overflow (SSO) Rule and the Capacity, Management,  
9 Operations, and Maintenance (CMOM) program requirements, the City included in the Master Plan a task  
10 to perform a preliminary CMOM program assessment. Integrating the CMOM assessment into the Master  
11 Plan provided a convenient mechanism for the City to tap into the expertise of the engineering  
12 consultants who were working on the Master Plan.

13 Gwinnett County, Georgia recently revised their storm water regulations in order to improve the water  
14 quality in the county's receiving streams and to meet the designated uses of the waters for fishing and  
15 drinking water supply (Chastant 2001). To meet that goal, a Watershed Protection Plan was developed,  
16 with the six major components being public education/participation, pollution prevention, development  
17 regulation, planning, engineering and construction, and maintenance. The current management strategy  
18 has been based on four key storm events: protection from major flooding events, overbank flooding bank  
19 protection from moderate events, channel bank protection for the 1-year storm, and water quality  
20 treatment for rainfall events of 1.2 inches and smaller. Atlanta's Clean Water Initiative was developed in  
21 order to coordinate water resource management for the Atlanta region and to generate the political  
22 momentum necessary for implementation (Green 2001). The task force recommended the creation of a  
23 "Metro Atlanta Water Planning District," which would be charged with developing watershed protection,  
24 wastewater, water supply and conservation plans. These recommendations and the District were enacted  
25 into law in 2001. In 1999, the City of Atlanta completed negotiations with the EPA and the Georgia  
26 Environmental Protection Division (EPD). The consent decree and an amendment required  
27 implementation of management, operation and maintenance (MOM) plans for CSO treatment facilities,  
28 Water Reclamation Centers (WRCs) and gravity wastewater collection and transmission systems (Griffin  
29 and Sukenik 2001). The MOM plans for the wastewater collection and transmission systems included  
30 emergency response plans; long- and short-term operation plans; maintenance plans; pump station  
31 evaluations; grease management plans; sewer mapping plans; safety and general training plans; and a  
32 short-term capacity certification plan. Hamid et al. (2001) presented an overview of Atlanta's Wastewater  
33 Systems Improvement Program. Three specific regulatory requirements drove this program: Senate Bill  
34 500 enforcing phosphorus level compliance at WRCs and CSO control facilities by December 2000; the  
35 CSO Consent Decree requiring completion of the long-term plan by 2007; and a second Consent Decree  
36 requiring WRCs, pumping stations and sewer improvements by 2014. The City's preferred option for the  
37 CSO plan included consolidated storage/conveyance tunnels, two CSO treatment facilities, and sewer  
38 separation in selected areas costing about \$1 billion. The total capital cost of the program was estimated  
39 between \$2.5 and 3 billion with a 14-year completion schedules. In November 1997, a Federal District  
40 judge ruled that the City of Atlanta's CSO discharges violated water quality standards for metals and fecal  
41 coliform. In addition, the Georgia EPD required compliance with water quality standards at the point of  
42 discharge for CSO facilities with no allowance for dilution (Richards and Kreutzberger 2001). Data is  
43 being collected in order to characterize water quality related to metals toxicity.

44 Clark et al. (2001a) combined hydrologic models of flood control and biotic models of ecologic risk with  
45 economic models of willingness-to-pay and psychological models of risk processing and planned behavior  
46 in order to evaluate these two alternative policy objectives. The findings reveal that flood risk exposure  
47 does influence the willingness-to-pay of local residents for a flood control project. Other important  
48 determinants include demographic factors, such as income, and attitudinal measures of the respondent.

49 The Department of Natural Resources in Queensland, Australia conducted the Queensland Water  
50 Recycling Strategy (QWRS) to determine future Government directions in water recycling (Gibson and  
51 Apostolidis 2001). This strategy considered the beneficial use of all waste streams such as domestic  
52 sewage, industrial and agricultural wastes, as well as urban stormwater. The Urban Development  
53 Corporation (UDC) in Japan developed a new system, a "Rainwater Recycle Sewer System" (Matsushita  
54 et al., 2001). This system is supported by "Rainwater Storage and Infiltration Technology (RSIT)" for new

1 town creation and urban renewal. The new system consisted of two elements: RSIT components based  
2 on Public-Private Partnership (PPP) and a stormwater drainage system. The private sector is responsible  
3 for the main part of RSIT, and the public sector is responsible for the stormwater drainage from the  
4 development area.

5 Augustenborg and Duke (2001) evaluated the effectiveness of current regulations for stormwater pollutant  
6 control from industrial facilities, and resulting efforts by the regulated community to reduce pollutants.  
7 Using the Notice of Intent filed by industries in three municipalities, trends and patterns of compliance  
8 were characterized. The research has evaluated the relative effectiveness of municipal programs at  
9 achieving the pollutant reduction goals of the storm water NPDES regulations. The wastewater and  
10 stormwater infrastructure in the U.S. was a significant financial investment; however, the maintenance of  
11 these systems has often been lacking (Kosco et al., 2001). Maintenance problems on private lands,  
12 particularly I/I from private sewer laterals, have been shown to represent a significant part of the water  
13 quality problems on a system. Many communities have aggressively addressed O&M issues of  
14 wastewater and stormwater systems on private property, with these programs backed up by local  
15 ordinances with penalties. Recently, Portland, Oregon revised their Erosion Control Program to have  
16 almost a zero tolerance for erosion and sediment leaving a site (Lauder 2001). The program also placed  
17 additional controls on construction site pollutants. The program required some new technology, but  
18 mostly management techniques and management issues were targeted. PDR Engineers, Inc. and the  
19 Louisville and Jefferson County Metropolitan Sewer District (MSD) produced the Watershed Approach to  
20 Environmentally Responsible Stewardship (WATERS) of Jefferson County report (Potempa et al., 2001).  
21 The report reviewed MSD's activities involving the Municipal Separate Storm Sewer System (MS4), the  
22 Combined Sewer Operational Plan (CSOP), and the Sanitary Sewer Overflow Abatement and Elimination  
23 Plan (SSOAE). The unique nature of the WATERS report is that the reader can view information from a  
24 programmatic and watershed approach. The Louisville and Jefferson County MSD prepares facility and  
25 watershed action plans with 20-year planning horizons (Wilson et al., 2001). This allows consideration of  
26 multiple factors, such as population projections, land use, environmental conditions, capital and operation  
27 and maintenance requirements, political issues, projections of economic conditions, etc. Using the multi-  
28 variable risk model has allowed MSD to assess the performance of a wastewater service area  
29 configuration over a range of views of the future.

30 The Federal Water Pollution Control Act Amendments of 1972 (a.k.a., the Clean Water Act (CWA))  
31 mandated that States lead aquatic resource protection and restoration activities. Congress directed the  
32 states to establish water quality standards for waterbodies based on use, identify waters that were not  
33 attaining those standards, and develop plans to improve the impaired waterbodies (Savage 2001). As  
34 Congress fashioned the statute, the EPA's role was to oversee the program. Although the Total Maximum  
35 Daily Loads (TMDLs) statutes (§303(d)) have not changed, the regulations promulgated by the USEPA  
36 regarding TMDLs have grown more complicated. While §303(d) and its regulations are a comprehensive  
37 approach to water management, they also have the potential to become a bureaucratic exercise in  
38 meeting deadlines and satisfying requirements that do not improve or protect the nation's water  
39 resources. Cited causes of impairment are often from sources that are not managed under Clean Water  
40 Act programs (Staveley et al., 2001), and are instead regulated under other statutes. These include  
41 sources such as atmospheric emissions, discharges of groundwater contaminated by past waste disposal  
42 practices, surface runoff from inadequately controlled landfills, historically contaminated in-place  
43 sediments, and the legal application of pesticides and herbicides. Thus, the possibility exists for overlap  
44 or conflict between the TMDL regulations and other statutes. Additional complications have arisen when  
45 comparing the provisions of the Safe Drinking Water Act (SDWA) and the Endangered Species Act (ESA)  
46 with the CWA.

#### 47 *Watershed Management*

48 Fulton County has begun addressing the surface water issues created by stormwater runoff (Ammons  
49 2001). The resulting Water Resource Management Plans have a 15-year program to improve the surface  
50 waters of Fulton County. In order to increase wastewater plant capacity and effluent discharge, the  
51 Georgia EPD required development and implementation of a watershed protection plan to address the  
52 "secondary" pollution impacts created by stormwater runoff from the development supported by the  
53 increased wastewater treatment capacity. The State of Georgia required watershed assessments to  
54 control nonpoint source pollution associated with degradation of surface water quality and non-attainment

1 of designated uses (Ahmad 2001). EPD required local governments to conduct watershed assessments  
2 as a part of the NPDES permitting process for wastewater treatment plant expansion. The study  
3 objectives included identifying the current aquatic health of the watershed; predicting the impacts from the  
4 proposed development; and identifying and assessing the performance of BMPs. The conclusions were  
5 that the creek currently meets all adopted and recommended water quality standards; the aquatic habitat  
6 show some degradation of biotic integrity; the planned level of development in the watershed will increase  
7 pollutant (and sediment) loads; the structural BMPs will mitigate the increased pollutant loads;  
8 nonstructural BMPs should be implemented as part of a long-term Water Resources Management Plan  
9 (WRMP); and a long-term water quality monitoring and sampling program should be implemented.

10 Batchelor and Rogers (2001), in response to the lack of improvement in some of the nation's waters,  
11 advocated finding smarter, more cost-effective ways of water quality and watershed restoration. Work in  
12 the Saginaw Bay Region showed that a point-source and non-point source trading program could be  
13 effective (Faeth, 1995). Two important points were made. (1) The current regulatory system needs to  
14 change to incentive-based systems. (2) Trading systems work for integrated water quality, air quality and  
15 natural resource programs. A trading program, on a watershed basis for trading, should provide major  
16 watershed restoration in the fastest time and for the least money. Bhimini et al. (2001) presented the  
17 findings, conclusions, and recommendations from a trading program for TMDLs on the Truckee River. In  
18 addition, the viability of a seasonal TMDL and a discussion on the phased approach towards developing  
19 pollutant trading permits were discussed. The development of the trading program included establishing  
20 the technical basis for modifying the TMDLs and WLAs and for identifying the optimum mix of structural  
21 and nonstructural BMPs. Kochoba and Wilber (2001) reviewed TMDLs in several states. The reviews  
22 indicated that TMDLs, and the resulting Load Allocations (LAs) and Wasteload Allocations (WLAs), often  
23 were based on limited data, unsound modeling and poor characterization of non-point source loads.  
24 Active involvement in the TMDL development process by contributing entities was advocated to help  
25 ensure that TMDLs are based on good science and result in reasonable, achievable WLAs and LAs.

26 As the first phase in its Integrated Resources Planning (IRP) process, the City of Los Angeles embarked  
27 on a program to restructure and redefine the future of water resources for the City and its neighbors  
28 (Boyle et al., 2001). The City developed the Integrated Plan for the Wastewater Program (IPWP), a set of  
29 policies to guide the next phases of wastewater facilities planning. The IPWP process used by the City  
30 integrated water supply, water conservation, water recycling, and stormwater management issues with  
31 wastewater facilities planning through a regional watershed approach. It also solicited and relied on public  
32 input in establishing planning-level policies. Philadelphia Water Department's Office of Watersheds was  
33 charged with integrating traditionally separate tasked programs, including the Combined Sewer Overflow  
34 (CSO) Program, the Stormwater Management Program, and the Source Water Protection Program  
35 (Dahme 2001). The goal was to maximize the resources allocated to these programs and to ensure the  
36 comprehensive achievement of each of their goals. The paper focused on the Darby-Cobbs Watershed  
37 Partnership.

38 The watershed storm water regulatory framework from the Rouge River National Wet Weather  
39 Demonstration Project (Rouge Project) in southeast Michigan was institutionalized in 1997 as a new  
40 statewide watershed-based general storm water National Pollutant Discharge Elimination System  
41 (NPDES) permit ("General Permit") (Cave et al., 2001). The General Permit required the immediate  
42 initiation of some activities such as illicit discharge elimination and participation in watershed  
43 management planning. The paper reported on the effectiveness of the subwatershed management plans  
44 developed under the General Permit in (1) contributing to the overall restoration of the Rouge River; (2)  
45 meeting the requirements of the state's Storm Water General Permit, and (3) satisfying the federal  
46 mandates contained in the Phase II Storm Water Regulations. Two years ago, most of the Rouge River  
47 Watershed communities became covered under the state's voluntary general stormwater permit program  
48 (Powell and Ball 2001). The permit required that the communities create a public education plan that  
49 included programs on the Rouge River, its impairments, stormwater pollution, and individual responsibility  
50 and stewardship. This paper discussed the initiatives and tools used by Wayne County's Rouge River  
51 National Wet Weather Demonstration Project and the involved communities to obtain public input during  
52 the subwatershed management planning process. In most cases, public meeting participation was the  
53 most difficult to predict and ultimately the least dependable, and many approaches were required.

1 Disaster mitigation related to floods and debris flows were discussed in the context of sustainable land  
2 and water management in Taiwan (Chen et al., 2001a). Engineering approaches to modify natural  
3 systems were contrasted with approaches that require changing human behavior. Mitigation success  
4 depended not only on the characteristics and magnitudes of the disasters but also on human responses  
5 related to socioeconomic, cultural, and political factors. Herricks (2001) discussed the renewed emphasis  
6 on watershed management programs that are based on ecosystem protection and restoration. The  
7 integration of ecosystem-based objectives into watershed management programs has demanded a  
8 substantial shift from past practices. This new paradigm integrated past management science and  
9 practice with ecosystem protection. This has assisted in the development of projects that produce needed  
10 ecosystem benefits.

11 Public involvement in watershed protection and stormwater management was reviewed by Cole (2001).  
12 Focused public involvement has encouraged residents to acknowledge individual responsibility for the  
13 impact of their actions on area waterways. It has also fostered voluntary participation in Watershed  
14 Management Plan recommendations. The paper discussed the objectives of a well-managed public  
15 involvement plan. Ames and Neilson (2001) presented an Internet-based, Bayesian Decision Network  
16 model to aid watershed stakeholders in collaborative decision-making. Bayesian Decision Networks  
17 (BDNs) have been shown to be a useful tool for diagramming the decision process, for describing  
18 relationships between variables, and for analyzing the anticipated effects of management decisions while  
19 still accounting for the associated uncertainties. An Internet-based BDN was described using the East  
20 Canyon watershed in Utah. Public participation was a key factor in the success of watershed  
21 management (Lu 2001). China Time's ten-year experience of promoting watershed planning was  
22 analyzed for public participation. Piasecki (2001) presented a methodology to compute sensitivities  
23 between benefits and costs that can be used to develop an efficient WLA procedure for CBOD/DO load  
24 scenarios. Steady state and unsteady flow scenarios were presented along with method sensitivities. The  
25 2-dimensional dynamic modeling approach permitted identification of a large number of temporally  
26 distributed sensitivities for sources along an estuary. The project described by Watkins and Paladino  
27 (2001) educated and mobilized watershed residents and other stakeholders to participate in the activities  
28 of a local watershed council. The underlying strategy was to engage stakeholders in discourse on  
29 watershed issues, as well as in activities that promote the decision-making skills necessary for  
30 sustainable watershed planning and management.

31 A watershed approach to Sanitary Sewer Overflow (SSO) management was applied for the Vallejo  
32 Sanitation and Flood Control District in Northern California (Dent et al., 2001). This watershed approach  
33 was used to estimate the total pollutant loads from SSOs, urban stormwater, and flows from the Napa  
34 River watershed. The results were then used to set permit limits for wet weather events. The watershed  
35 approach combined monitoring and modeling techniques to estimate the water quality improvements for a  
36 variety of SSO elimination activities. A joint stormwater initiative for the City of Lincoln and the Lower  
37 Platte South Natural Resources District was successful in achieving three goals: adoption of  
38 comprehensive stormwater ordinances for new development; implementation of revised, comprehensive  
39 design criteria; and completion of the City's first stormwater basin plan (Masters et al., 2001). Ordinance  
40 and design standard revisions addressed maintenance issues for stormwater storage facilities, erosion  
41 and sediment control, localized flooding, water quality, and the specific criteria for meeting objectives in  
42 these areas. Holmberg et al. (2001) reviewed the need for municipalities to balance establishing peak  
43 flow standards with these other issues: dynamic sewer flows resulting mostly from I/I originating on  
44 private property; capital funding; risks of public health and environmental effects from SSOs; and threats  
45 of third-party citizen lawsuits. The Water Works and Sanitary Sewer Board of the City of Montgomery  
46 (Board) advocated the watershed approach to manage collection systems. The Board used the  
47 watershed approach in two of its three sewer basins – the Towassa and the Catoma Basins. The Board  
48 also recently used the watershed approach process to set peak flows in the Towassa Basin. Lewis and  
49 Hilson (2001) explained how environmental benefits were optimized against cost through integrated  
50 catchment modeling in North West England. These solutions were obtained from detailed modeling of the  
51 wastewater networks, along with the modeling of the wastewater treatment works and the receiving  
52 waters. The stormwater management system in Calgary is a zero discharge system incorporating  
53 sedimentation, biological uptake/treatment and irrigation for disposal of stormwater runoff (MacKenzie  
54 and Dumont 2001). The system was designed and implemented, and is in successful operation. The zero  
55 discharge stormwater system allowed the development area to proceed without off-site treatment

1 facilities. The system controlled/mitigated the effects of stormwater runoff on receiving waters. Continuous  
2 simulation, using long-term records of precipitation, temperature, and evaporation combined with  
3 estimates of plant moisture requirements formed the basis for the design of the system.

4 McGrath (2001) discussed the growing need to forecast land use changes and urbanization patterns over  
5 longer time frames so that regional environmental and hydrologic consequences can be forecasted and  
6 the consequences of different development scenarios compared. Obstacles to achieving such forecasting  
7 capability included the integration of spatial environmental/hydrological and spatial economic models. In  
8 addition, the econometric problems associated with estimating spatial economic changes were reviewed.  
9 The recognized difficulties in combining the relevant disciplines to produce accurate long-range forecasts  
10 of land use change were discussed. Ecological impairment and flooding caused by urbanization was  
11 expressed numerically by calculating the risks throughout the watershed (floodplain) and along the main  
12 stems of the streams (Novotny et al., 2001). The methodologies for ascertaining the risks in the  
13 Geographical Information Systems (GIS) environment were described. The objectives of urban flood  
14 controls and ecological preservation/ restoration of urban waters often have conflicted. A solution to this  
15 may be achieved by linking the risks to the concepts of risk communication, risk perception, and public  
16 willingness to pay for projects with ecological restoration and ecologically sustainable flood control.  
17 Reeves et al. (2001) developed a method that would incorporate the effects of both chemical and physical  
18 parameters on aquatic life (Reeves et al., 2001). The resulting method, termed the Integrated Impact  
19 Analysis method, used existing analytical methods, including Principle Components Analysis, All Possible  
20 Regressions, and Chi-Square Automatic Interaction Detection (CHAID), to identify key variables. Non-  
21 linear interactions between the key variables were identified using a three-dimensional modeling program.  
22 The method was refined using three datasets (Santa Ana River in California, South Platte River in  
23 Colorado, and Cuyahoga River in Ohio). Webster et al. (2001) considered the problems of developing a  
24 perceptive and efficient rehabilitation strategy for a large urbanized catchment, specifically the River  
25 Tame in the West Midlands. The following were considered: (a) rainfall-runoff relationships, (b) water  
26 quality and ecological status, and (c) the impact of various management strategies. Spalding and  
27 Sweeney (2001) advocated the use of the Internet by wastewater utilities to achieve closer customer and  
28 production integration. A "Digital Utility" approach using the Internet to create new, robust, customer-  
29 oriented applications was described.

30 In Cleveland, Ohio, channel erosion and flooding, water quality, and aquatic life and habitat were  
31 evaluated (Yingling 2001). The purpose of both the Mill Creek and Doan Brook Watershed Studies was to  
32 develop comprehensive approaches for controlling wet-weather impacts on the respective streams. The  
33 cost and effort involved in actually performing the watershed studies, as well as the potential for overall  
34 cost savings and creativity in developing final solutions, were reviewed. Although watershed studies  
35 require a significant investment in data collection and in public involvement, the resulting solutions were  
36 typically more cost-effective and accepted by the public. Waterific, an interactive science program about  
37 water, was created to educate the public about water-related issues (Ziegler et al., 2001). The focus of  
38 Waterific was to enhance environmental awareness, education and celebration. In addition, the program  
39 is a fun, hands-on activity that meets the core content required by the Kentucky Department of Education  
40 for sixth-grade science. By pooling talent from various water quality agencies, the school students saw  
41 first-hand how all the agencies work together and independently to maintain the water quality in Northern  
42 Kentucky.

### 43 *Permitting*

44 Under Phase I of the NPDES permit requirement, permits were required for stormwater discharges  
45 associated with industrial activity (Daniel 2001). Typically these discharges were material handling and  
46 storage areas at certain industries. Authorized dischargers were required to develop and implement  
47 stormwater pollution prevention plans to prevent the discharge of pollutants in runoff. Pollution prevention  
48 has been the key for stormwater discharges associated with industrial activities (Duke 2001). However,  
49 evidence suggests the pollution prevention approach is widely failing. In targeted case studies, evaluation  
50 of chemical constituents in runoff shows no evidence of pollutant reduction over time. Effective  
51 implementation of the pollution prevention approach may require more resources by regulators compared  
52 to classical command-and-control approaches, but instead agencies have devoted far fewer resources  
53 and relied on voluntary compliance. Therefore, it is important for the Industrial community to understand  
54 what these regulations are trying to accomplish and how these regulations will be locally specific. The

1 paper by Gates and Resiak (2001) discussed when the State of Indiana will implement the Phase II  
2 regulations, who will need to apply for Phase II permit coverage, and what requirements will be in those  
3 permits. These were illustrated by a specific case study.

4 Georgia EPD and the Board of Natural Resources have to decide how and who should do watershed  
5 protection (Scarborough 2001). Georgia concluded that POTWs should be the responsible party since the  
6 County/City controls land use in Georgia. The purpose of the Gwinnett County Watershed Assessment  
7 was to evaluate stream conditions within the County and recommend a watershed protection strategy to  
8 improve the streams which were not meeting the designated use. Patel (2001) provided a general  
9 background and state perspective for municipalities to approach the regulatory requirements for the new  
10 EPA Phase II Stormwater regulations. The description of the proposed Pennsylvania State Strategy  
11 summarized what the Phase II regulations will require of the regulated communities, and answered basic  
12 questions of who is affected, what is required, when permit applications are to be submitted, what  
13 permitting options are available, what types of permits may be available, what minimum information on  
14 permit applications may be necessary, and what, if any, funding assistance may be available. The CMOM  
15 approach in the SSO Rule will require “a dynamic system management framework that encourages  
16 evaluating and prioritizing efforts to identify and correct performance-limiting situations in the collection  
17 system” (Ruggard 2001). The goal of the ultimate elimination of any type of overflow from the sanitary  
18 sewer system was established at the Stege Sanitary District in 1996. The paper reviewed the  
19 improvements to the District’s management, engineering, maintenance and data management systems in  
20 order to achieve that goal.

#### 21 *Cost Analysis and Financing*

22 Stormwater controls and Best Management Practices (BMPs) were evaluated within a land development  
23 context (Sample et al., 2001a). Costs were developed from published literature and standard cost  
24 estimation guides. Data gaps and research needs were then explored. Many communities across the  
25 country have developed user-charges – and their delivery vehicles, stormwater utilities – as the primary  
26 funding source for expanded maintenance and new infrastructure (Rose 2001). With the implementation  
27 of a new funding source (e.g., user fee or special assessment) for stormwater management programs, a  
28 municipality imposes a new financial burden on the public (McClelland 2001). Therefore, the key to a  
29 successful stormwater utility implementation is public approval. The article presented methods for public  
30 education and approval, particularly the shift in focus from the fees to the stormwater service the new  
31 revenue will provide. Sound rate methodology should draw upon key concepts from public finance, utility  
32 theory, economics, law and engineering science. Most rate structures have been built around the “runoff”  
33 theory. Within this “runoff” theory are four related but different basic models: (1) the “impervious area”  
34 model, (2) the “weighted pervious plus impervious area” model, (3) the “land use” or “density” model, and  
35 (4) the newly emerging “pollutant loading” model. Fitting a basic model to a community is largely a matter  
36 of community political context.

37 Hall et al. (2001a and b) reviewed EPA’s development of requirements applicable to design and  
38 permitting of wet-weather flow management facilities. These include issuance of various combined sewer  
39 overflow policies, development of regulations to require elimination of SSOs, enforcement actions against  
40 municipal entities, objections to state program permits; informal guidance to Regional Offices, and  
41 Regional Office initiatives. The paper reviewed the major inconsistencies in current program  
42 implementation, evaluated the applicable federal regulations and permit approaches, and recommended  
43 broadly, how communities with SSOs and CSOs should approach state and federal agencies to resolve  
44 these matters. Salo (2001) reported on the comprehensive assessment of the City of Atlanta’s water and  
45 wastewater operations. Internal and external management approaches were considered after it was  
46 found that a substantial rate increase would be required to fund a Capital Improvement Program. The  
47 adopted plan combined approaches through contract operation of the entire Water System, contract  
48 operation of the largest wastewater treatment plant, and reengineering of the other City wastewater and  
49 sewer operations.

50 Present worth is often used to select among alternatives in the public works arena (Bate et al., 2001).  
51 This is adequate if the options are technically equal and operations and maintenance costs are  
52 controlling, but it overlooks are intangible criterion such as social impacts, meeting the design criteria,  
53 construction, traffic and utility impacts, constructability, as well as service life and reliability. “Paired

1 comparisons” allow comparisons of intangible criterion that impact the project. Case histories illustrated  
2 the use of the “paired comparison” process. Booth et al. (2001) reviewed a survey of residents in the Oak  
3 Creek and Menomonee River watersheds, both located in Milwaukee County, Wisconsin to determine the  
4 willingness to support ecological restoration of urban streams. One purpose of the project was to assess  
5 the willingness to pay (WTP) of urban watershed residents for urban stream restoration and to identify the  
6 underlying economic, psychological, and social motivations for WTP. The basic hypothesis tested here  
7 was that psychological variables, environmental attitudes, and ethical values are more important than  
8 strictly economic phenomenon.

9 In the future, systematic and ongoing asset management programs are expected to influence evaluations  
10 by bond rating agencies, budget allocations, decisions by enforcement authorities, and acceptance of rate  
11 increases (Morgan and Wagner 2001). This approach incorporates life-cycle analysis, longer planning  
12 horizons and more intensive tracking of asset conditions. Of the nine primary program components in the  
13 CMOM/SSO guidance, most are related directly or indirectly to asset management activities. The  
14 privatized operation of the Milwaukee Wisconsin, sewerage facilities has lowered costs of operation (\$14  
15 million per year) and increased service performance (Tobel and Jankowski 2001). Additional benefits  
16 include increased asset and performance accountability and state-of-the art operations and maintenance  
17 (O&M) technologies. The Kansas City Missouri Water Services Department (WSD) developed a  
18 Competitive Business Plan (CBP) (Salo and Turner 2001). The CBP identified how WSD will improve  
19 service levels and minimize annual operating costs. Implementation of this CBP will require significant  
20 additional capital investment. O&M savings are expected to be between \$129 and \$217 million over the  
21 next 10 years.

## 23 **CONTROL AND TREATMENT TECHNOLOGY**

### 24 *General*

25 Growing demands on drainage still challenge designers with respect to runoff quantity and quality;  
26 landscape aesthetics, ecology and beneficial uses; and operation of existing urban wastewater systems  
27 (Chocat et al., 2001). Integrated approaches, optimal operation of the existing infrastructure, advanced  
28 pollution and runoff source controls, improved resilience of receiving waters, and adaptive water  
29 management can achieve further advances in water quality protection. Specific research needs include  
30 new technologies and strategies for stormwater management, advanced treatment of urban wet-weather  
31 effluents, and tools for analysis and operation of drainage systems. Smith et al. (2001a) investigated a  
32 range of process technologies to assess their suitability for the treatment of different water sources for  
33 non-potable reuse. For this study, a large scale water reclamation evaluation and demonstration facility  
34 was constructed at the Millennium Dome, London, UK. Greywater, rainwater and poor quality  
35 groundwater were treated by a combination of processes including biological oxidation, constructed  
36 wetlands, chemical oxidation, adsorption, ultrafiltration and reverse osmosis. Chemical and  
37 bacteriological results for each process were presented and discussed.

38 The WERF report on controlling pollution at its source documented the application of effectiveness  
39 measurement tools at demonstration projects that controlled wastewater and stormwater pollution at its  
40 source (WERF 2001a). The report described the demonstration projects, the tools tested, identified the  
41 costs involved, and assessed the feasibility of measuring a source control program’s impact. The  
42 Minnehaha Creek Watershed District has funded a multi-component project focused on improving the  
43 water quality in Lake Nokomis (part of the chain of lakes in Minnesota) (Hettiarachchi et al., 2001). The  
44 project components included constructed wetlands and grit chambers to treat runoff from surrounding  
45 residential neighborhoods, an inflatable weir to prevent Minnehaha Creek water from flowing back into the  
46 lake, and rough fish removal and alum treatment to reduce the internal phosphorus loading.

### 47 *Drainage Design and Hydraulics*

48 As part of the Clean Water Act, EPA required local governments to control urban storm water runoff  
49 (Ports 2001). asked the General Accounting Office (GAO) to report on the amount of runoff from urban  
50 areas, particularly from roads and other impervious surfaces, and its effects on water quality, and to  
51 perform an overall evaluation of the urban runoff control programs that federal regulations require,  
52 including their cost and effectiveness. The results from the GAO evaluation were summarized in a report

1 titled, "WATER QUALITY, Better Data and Evaluation of Urban Runoff Programs Needed to Assess  
2 Effectiveness." The paper by Pazwash and Boswell (2001) presented an overview of design methodology  
3 of various stormwater management systems, including detention basins, wet ponds, infiltration beds and  
4 underground detention/retention chambers. It discussed shortcomings in design and recommended  
5 improvements. The paper also described differences in local and state stormwater management  
6 regulations in New Jersey and offered suggestions for unifying the regulations.

7 Parsons Engineering Science, Inc., as part of an EPA project, reviewed industry and state practices for  
8 sizing both new and rehabilitated sanitary sewers (Mauro et al., 2001). Several new tools for estimating  
9 peak flows with reflect current technology and water use habits were identified. The research has found  
10 that peak flows in sanitary sewers depended on a complex set of variables. Tools used by communities to  
11 evaluate the operation of sewer systems were identified through interviews with experienced  
12 practitioners, research on federal and local regulations and industry guidance documents. The  
13 wastewater industry has been performing detailed I/I studies for the past 25 years (Lyon and Nelson  
14 2001). The lessons learned can be applied to sanitary sewer design criteria to reduce the future  
15 occurrence of SSOs. Data from I/I studies for 20 wastewater agencies in six states (Kansas, Kentucky,  
16 Minnesota, Missouri, Ohio, and Texas) were compared, and guidelines for appropriate sewer design  
17 criteria were developed. Due to the complex hydraulics in the vicinity of the Jackson Pike WWTP and as  
18 part of the West Columbus (Ohio) Local Protection Project (WCLPP), a detailed dynamic model was  
19 developed to assess the sewer system performance under different operating conditions (El-Hosseiny et  
20 al., 2001). The model used the sewershed approach to maximize system storage without affecting the  
21 current services and to satisfy the regulations. The sewershed approach and the developed model  
22 allowed the determination of the optimum operating conditions by maximizing in-line storage while  
23 meeting all required constraints (e.g., overflow, pumping capacity, control flow devices, sewer capacity).  
24 The paper by McConico et al. (2001b) described a mechanical flushing system located in the confined  
25 space of the CSO Outfall Structure for the Shockoe Creek Basin in Richmond, Virginia. Over time,  
26 deposition of solids in the twin river crossing has reduced the flow capacity of the inverted siphon  
27 pipelines, reducing transfer capacity to the wastewater treatment plant. The mechanical flushing system  
28 was designed to store and release a large volume of wastewater to flush the twin river crossing with a  
29 high volume, high velocity stream while avoiding accidental releases of dry weather overflow to the river.

30 Many areas in the United States have established stormwater detention pond ordinances that require the  
31 postdeveloped outflow from a site to discharge at a rate equal to or less than the predeveloped peak flow  
32 rate (Glazner 2001). The impacts of increased stormwater runoff volume due to development were  
33 reviewed. Using a hypothetical catchment in Chicago, Illinois, Guo (2001b) compared three different  
34 approaches for the hydrologic design of flood control detention ponds that service urban catchments: (1)  
35 design storm approach; (2) continuous simulation approach; and (3) analytical probabilistic approach.  
36 While all three approaches generated similar results, the results from the design storm approach were  
37 shown to vary by 40% to 50% depending on the choice of models and design storms. The results further  
38 verified the suitability of the analytical probabilistic approach for the hydrologic design of urban flood  
39 control detention ponds. Hsieh et al. (2001) evaluated the flood mitigation performance of various long-  
40 term regulated plans (protecting banks, pumping stations, diversion works, and retention reservoirs) for  
41 the middle-upstream of Kee-Lung River Basin in northern Taiwan through simulations of both 1-D  
42 unsteady flow and 2-D overland flow. The models evaluated the flood mitigation effect of each measure  
43 based on three criteria, including flood stages, runoff peaks, and inundation depths and ranges. The  
44 results showed that none of proposed regulated plans was best at resolving the flood and inundation  
45 problem. Storm water detention pond design is frequently part of each site or land development project  
46 (Ovcharovichova 2001). One critical aspect of the design is the determination of the boundaries of  
47 drainage areas for both existing and future conditions. The paper compared two different models of the  
48 same pond, with the only difference being in the existing and future drainage area delineation.  
49 Consequently, the results are different. The paper suggested which approach should be preferred to  
50 simulate the basin as it truly functions in the field. The paper by Wong (2001) contended that for drainage  
51 design to be based on a consistent theory, the kinematic wave time of travel formula for channel flow  
52 ought to be used in conjunction with the kinematic wave overland time of concentration formula. The  
53 paper contains the kinematic wave time of travel formulas for channels of seven different cross sections.  
54 A model of a hydrologically isolated section of the M6 motorway in the United Kingdom was used to  
55 calculate the times of travel in a channel with a vertical curb section for rainfall intensities ranging from 20

1 to 100 mm h<sup>-1</sup>. A comparison of the time of travel estimates from the computer model, and from the time  
2 of travel formula, shows that the differences are less than 5%. The derived time of travel formulas could  
3 be used to design drainage channels whose properties fit the kinematic wave approximation.

#### 4 *Stormwater Best Management Practices (BMPs)*

5 BMPs can range from management operations (such as street sweeping or reducing pesticides used on  
6 urban lawns) to structural treatment options (such as detention/retention ponds, swales, filter/buffer strips  
7 and constructed wetlands) (Sullivan and Borst 2001). This paper focused on structural BMPs and  
8 reviewed the state of the knowledge, the unknowns, and research programs being undertaken by the  
9 U.S. Environmental Protection Agency and other key organizations to address the unknowns.

10 The final report will document the effectiveness of these BMPs to improve water quality and will address  
11 BMP stability, longevity, and operation and maintenance issues. BMP manuals have been developed that  
12 address the control of urban runoff to protect receiving water quality (Roesner et al., 2001). Investigations  
13 of both design practices and effectiveness revealed that there is a lack of knowledge in the scientific and  
14 engineering community about what constitutes a properly designed BMP and what the BMP should  
15 achieve. This paper discussed the state-of-practice in BMP design in the US and pointed out the  
16 strengths and weaknesses with respect to water protection. An approach to design criteria development  
17 for a wide variety of climatologic, topologic, and geologic conditions was recommended to protect  
18 receiving waters systems. Strecker et al. (2001) reported on the EPA-funded cooperative research  
19 program with the ASCE to develop a more useful set of data on the effectiveness of stormwater BMPs in  
20 urban development. The paper described the comparability problems encountered between different BMP  
21 effectiveness studies and the considerations that affect data transferability, such as methods used for  
22 determining efficiency and statistical significance. Finally, it recommended that effluent quality be used to  
23 measure BMP efficiency. In the paper by Yu and Zhen (2001), a methodology is developed to assist in  
24 the determination of BMPs placement strategies at the watershed level. The Agricultural Non-Point  
25 Source Pollution Model 2001 (AGNPS 2001) developed by USDA, was used for BMP placement analysis,  
26 and the relative effectiveness of BMPs at three different spatial placement levels, i.e., on-site, sub-  
27 regional and regional levels, were compared. Based on the model simulation results, a BMP placement  
28 optimization approach was developed to determine a most cost-effective BMP placement strategy at the  
29 watershed scale.

30 Development projects in the high-altitude mountain environment of the Rocky Mountains such as  
31 Colorado often require innovative best management practices (BMPs) due to challenging runoff  
32 conditions, the relatively short growing season, vegetation and wildlife habitat considerations, and the  
33 high level of water quality of receiving waters (Earles and Jones 2001a). This paper described a variety of  
34 BMPs and stormwater/dewatering discharge management strategies that have been successfully  
35 employed on development projects. Case studies were presented for addressing runoff from snowmelt;  
36 shallow groundwater; soil erodibility, mobilization, and suspension; water chemistry; and regulatory  
37 requirements related to water quality and wetlands protection, and included examples of structural BMPs.  
38 Yamada et al. (2001a) analyzed the mass balance of pollutants during both dry periods and storm events  
39 on Lake Biwa and discussed the effects of pollutant removal systems, land use planning and new  
40 drainage systems by simulation. The project included estimation of influent pollutant loadings from  
41 existing data, collection of additional samples from road surfaces, house roofs and parking lots, and  
42 evaluation of ongoing BMP projects. Hambridge and Stein (2001) studied a 33-acre lake in Duke Power  
43 State Park where lake water quality is deteriorating. The EPA "Simple Method" screening model was  
44 used to estimate the contribution of pollutants from each land use category, and a goal for BMPs to  
45 reduce phosphorus loadings by 34% was determined. Restoration options included dredging; developing  
46 an in-lake wetland; and carp eradication. Watershed BMPs included regional stormwater treatment;  
47 parking lot bioretention; wetland treatment at a local high school; strengthening stream buffer restrictions;  
48 agricultural BMPs; education; and septic maintenance programs. In Onondaga County, an Amended  
49 Consent Judgment required the County to perform a NPS environmental benefit project (EPB) in  
50 Onondaga Lake watershed (LaGorga et al., 2001). BMPs were implemented on three farms and at two  
51 urban sites in the Onondaga Lake watershed. The Staten Island Bluebelt Program represents New York  
52 City's first large-scale use of stormwater BMPs to alleviate chronic flooding and provide drainage  
53 infrastructure (Vokral et al., 2001). The BMPs used by the t Program included constructed wetlands with  
54 extended detention, stilling basins, underground sand filters, and meandering streams. These BMPs were

1 effective on a large scale (even preventing flooding during Hurricane Floyd). The enhanced wetlands and  
2 improved water quality greatly promoted biodiversity. Public support was enthusiastic and valuable  
3 educational/recreational resources and cost-effective drainage infrastructure resulted.

4 Low Impact Development (LID) combines hydrologically-functional site designs with pollution prevention  
5 measures as compensation for land development impacts on receiving waters (Clar 2001). This paper  
6 summarized the results of a number of demonstration projects, and introduced a strategic framework for  
7 the application of LID technology to ultra urban areas. It reviewed the hydrologic characteristics of ultra  
8 urban areas and related them to identified stormwater management goals in ultra urban areas. LID  
9 combines conservation strategies, distributed micro-scale source control BMPs, and pollution prevention  
10 to control the volume and peak runoff rate, and to treat the pollutants (Weinstein et al. 2001). LID  
11 practices can be incorporated into buildings, sidewalks, streets and landscaping. Because they are small  
12 scale, the issues associated with conventional large-scale, end-of-pipe controls, such as large-scale  
13 traffic and property disruptions, utility conflicts and large capital costs, can be avoided. The paper  
14 documented the issues and efforts of a multi-year monitoring and construction effort of the Maryland State  
15 Highway Administration (MSHA) to determine the effectiveness of LID at addressing regulatory and  
16 ecological protection goals for highway construction. The development and use of environmentally  
17 sensitive construction materials as a low-cost component to stormwater management has gained interest  
18 recently (Pitt and Lalor 2001). However, there is little data for specific alternative building materials,  
19 although information exists targeting selected sources, especially the role of roof runoff as a significant  
20 source of zinc and other metals. Relative pollutant contributions from construction materials themselves  
21 are also a concern that has not been adequately addressed. Due to the common use of these materials in  
22 the urban environment, material substitution would seem a good place to start in implementing source  
23 reduction.

24 To assess the effectiveness of sedimentation and erosion controls during highway construction, the  
25 impact on water quality in adjacent wetlands was monitored (Huang and Ehrlich 2001). Downstream  
26 measurements were equivalent statistically to upstream measurements except once when erosion  
27 controls were neglected and when culverts were constructed. Attention to sedimentation and erosion  
28 controls and seasonal scheduling of highway construction were advocated to protect adjacent wetlands.  
29 Clopper et al. (2001) reported on an erosion control study that examined the benefits of two different  
30 erosion control mulches; blown straw and a manufactured biodegradable erosion control blanket.  
31 Sprinkler-type simulators were used to create rainfalls having intensities of 2, 4, and 6 in/hr. The collected  
32 data indicated that some surface cover treatments consistently reduced soil losses, while others can  
33 actually increase soil losses under some test conditions. They concluded that the Revised Universal Soil  
34 Loss Equation can be used to estimate the benefits of mulches at construction sites. Shammaa and Zhu  
35 (2001) presented a state-of-the-art review of TSS removal techniques. Three main techniques were  
36 reviewed: infiltration, filtration and detention. Infiltration trenches, infiltration basins and porous pavements  
37 were the common infiltration practices. Filtration systems included filter strips, grassed swales and media  
38 filters. Wet and dry detention ponds (including polymer-assisted ponds) and constructed wetlands were  
39 the most common detention practices. The function, performance and suitability of each technique were  
40 discussed, and a comprehensive review was provided to guide the selection of a suitable TSS control  
41 technique. Levee sump systems have been used by many riverine communities for temporary storage of  
42 urban wet weather flows (Smith et al., 2001d). This paper presented a case study that demonstrated a  
43 procedure for assessing the hydraulic performance of flood control sumps in an urban watershed. A  
44 hydrologic modeling package was used to estimate the flow hydrograph for each outfall as part of the flow  
45 balance for the sump. In addition, these sumps may function as sedimentation basins. Yu et al. (2001b)  
46 monitored several ultra-urban BMPs: manhole-type treatment structures (such as the Stormceptor), a  
47 bioretention area, and the Vortechs Stormwater Treatment System. Resuspension of sediment during  
48 large storm events has been a concern for these BMPs. A cost analysis showed that bioretention might  
49 be the most cost-effective in terms of cost per unit pollutant removal. However, the bioretention area may  
50 export pollutants before the soil-plant system stabilizes.

51 NPDES Stormwater Phase II and TMDL regulations have placed additional pressure on industrial  
52 facilities to reduce stormwater pollutant levels by implementing source controls and stormwater treatment  
53 (Mas and Curtis 2001). This paper presented an overview of the assessment and planning phases of  
54 industrial stormwater treatment evaluations, with a focus on structural control measures for removal of  
55 dissolved metals and other industrial stormwater pollutants common to the Northeast U.S. Chang and

1 Duke (2001a) evaluated actions taken by auto dismantling facilities in the Los Angeles, California, region  
2 to comply with industrial stormwater discharge regulations. The research evaluated a sample of  
3 complying facilities, and involved stormwater sampling at a smaller number of case study facilities. The  
4 study found that a large proportion have measured effluent concentrations that exceed U.S. guidelines for  
5 stormwater. Estimates of pollutant loads contributed by the dismantling industry appear substantial. The  
6 Ford Rouge Center complex on the Rouge River is undergoing dramatic revitalization, including  
7 substantial redevelopment of the existing plant structures and the addition of several new buildings  
8 (Houston et al., 2001). Cahill Associates developed a stormwater master plan that provides providing the  
9 facility with onsite stormwater storage and water quality improvement for the smaller, more frequent  
10 storms. A general strategy of retention and treatment through water quality swales was adopted. The two-  
11 year storm volume was used as the minimum storage volume required for each system.

12 The Dubai International Airport expansion plans required a stormwater management strategy that met  
13 international regulatory authority requirements, addressed time-sensitive milestones, and remained cost-  
14 effective (Darnell et al., 2001a). The stormwater management system must achieve the minimum  
15 drainage requirements and also provide a multitude of management options that could address the  
16 unpredictability of desert-climate storms. When modifications were done to the Camarillo (Ventura  
17 County), California, Airport in 1997, a high efficiency oil-water separator was installed to treat the  
18 stormwater runoff from the area where the refueling trucks were filled (Mohr et al., 2001). The system  
19 included a precast concrete separator vessel with multiple-angle enhanced gravity separator plate packs.  
20 Performance tests were performed on the system as part of the acceptance procedure. The results were  
21 presented along with the results of three year's operations and maintenance.

#### 22 *Public Works Practices*

23 Because more than 780,000 tonnes of solids washed is washed into the drainage system in South Africa,  
24 the Water Research Commission of South Africa and the Cape Metropolitan Council are funding a four  
25 year investigation into the reduction of urban litter in the drainage systems through the development of  
26 catchment specific litter management plans (Armitage et al., 2001). The results of the litter audits from  
27 eight catchments will measure the effectiveness of the various litter management strategies. The  
28 California Department of Transportation (Caltrans) conducted a 2-year litter management pilot study in  
29 the Los Angeles area to investigate the characteristics of highway litter and the effectiveness of BMPs for  
30 removing the litter (Lippner et al., 2001). Half the catchments were treated with one of five BMPs; the  
31 others were controls. The BMPs tested were increased street sweeping frequency, increased frequency  
32 of manual litter pickup, a modified drain inlet, a bicycle grate inlet, and a litter inlet deflector (LID). Roughly  
33 half the freeway storm water litter was paper, plastic, and Styrofoam. Except for cigarette butts, the  
34 origins of most litter could not be identified because of its small size. Of the five BMPs tested, only  
35 increased litter pickup and the modified drain inlet demonstrated some apparent reduction of litter in  
36 stormwater runoff, although the data were highly variable.

37 Some people have advocated annually removing sediment, vegetation and litter from drain inlet vaults as  
38 a best management practice to improve the quality of Caltrans run-off before it enters the receiving waters  
39 (Dammel et al., 2001). In response, Caltrans implemented an annual drain inlet inspection and cleaning  
40 program in selected urban areas, and conducted the Drain Inlet Cleaning Efficiency (DICE) Study to  
41 evaluate if this practice improved effluent water quality. Irgang et al. (2001) evaluated the effect of catch  
42 basin cleaning on stormwater quality. Catch basins within two of the four drainage areas were cleaned at  
43 the beginning of the study, while those within the other two areas were not cleaned. Pollutant  
44 concentrations and runoff loadings were compared between the two areas. It was observed that fine  
45 particle deposits remaining in catch basins after cleaning could cause higher pollutant concentrations and  
46 loadings for several months when compared to control areas where catch basins were not cleaned.  
47 Untreated stormwater runoff reaches Santa Monica Bay (Los Angeles, California) primarily through catch  
48 basins or inserts to storm drains that terminate at the beach or in shallow coastal areas (Lau et al., 2001).  
49 Commercially available drain inlet devices for pollutant capture exist but few have been evaluated by  
50 independent parties in full-scale applications. Laboratory- and full-scale tests of inserts were conducted to  
51 evaluate their ability to remove trash and debris, suspended solids and oil and grease in stormwaters,  
52 with the results providing a basis for future insert development and application. The performance of Drain  
53 Inlet Inserts (Fossil Filter and StreamGuard) and an oil/water separator in treating runoff from four  
54 California Department of Transportation (Caltrans) maintenance stations was evaluated (Othmer et al.,

1 2001). Drain Inlet Insert results to date showed that reductions in metals, hydrocarbons, and solids were  
2 consistent with expectations; however, frequent flow bypass required more maintenance than anticipated.  
3 Oil/water separator results showed no discernable difference between influent and effluent hydrocarbon  
4 concentrations at the low levels measured.

5 Newman (2001) described an analytical framework for the design and/or analysis of baffles to reduce  
6 floatables discharges from CSOs. This simple analytical framework, which is supported with a  
7 spreadsheet model, was compared to its predecessors and its advantages illustrated. The advantages  
8 included ease of use, improved applicability to typical installation configurations, and refined analyses of  
9 floatables-removal mechanisms. Model results were compared to previous results and to available  
10 laboratory test data for four test cases.

11 Interest in urban stream restoration has grown (Hession 2001). However, a scientific basis for restoring  
12 urbanized streams currently does not exist, although it is known that riparian vegetation along streams  
13 significantly impacted stream channel morphology, which in turn influences aquatic ecosystem structure  
14 and function. Watershed urbanization also has a significant, but typically conflicting, influence on channel  
15 morphology and aquatic habitat.

#### 16 *Infiltration and Biofiltration*

17 The use of porous bituminous pavement for infiltration as a method to reduce the peak discharge and  
18 runoff volume into a receiving water was examined by Adams et al. (2001). In addition, this paper reviews  
19 the literature for infiltration meadows, trenches, swales, porous concrete sidewalks, bioretention gardens,  
20 etc. The design, installation, and performance of these methods at actual sites, "lessons learned," and the  
21 effectiveness of these infiltration methods were reviewed. Infiltration of stormwater in the southeast of  
22 France was studied by Bardin et al. (2001) at the Venissieux infiltration basin that drains a 380-ha  
23 industrial catchment. The study quantified the effects of the infiltration system in terms of pollutant  
24 transport in the groundwater system, including pollutant removal performance of the basin and the  
25 pretreatment devices. In-situ performance monitoring of an infiltration system that collected the runoff of a  
26 school roof and paved area was reported by Abbott and Comino-Mateos (2001). The resulting data was  
27 used to verify the typical design procedures for these infiltration systems. Govindaraju et al. (2001)  
28 investigated the difficulty of modeling infiltration through soils that had spatially varying saturated  
29 hydraulic conductivities, but which can be represented by a homogeneous correlated lognormal random  
30 field. The Green-Ampt equation described the infiltration at the local scale. Approximate expressions  
31 based on a series expansion and a parameterization of the local cumulative infiltration were also  
32 presented for describing the ensemble-averaged field-scale infiltration.

33 Gharabaghi et al. (2001) monitored the sediment removal efficiencies of vegetative filter strips. Fifty  
34 percent removal efficiencies were seen when the flow length was 2.44 m and were increased to 98%  
35 when the flowpath length was 20 m. However, small-sized particulates were not effectively removed in  
36 the grass strips. Improved removal efficiency of very-fine sediments was achieved through the installation  
37 of a drainage system (e.g. a French drain) to increase infiltration. Reinforcement of vegetation with  
38 various geosynthetic products reduced flow channelization and short-circuiting. Yu et al. (2001a) field  
39 tested grass swales in Virginia and Taiwan. Average pollutant removal efficiencies varied from 14 to 99%  
40 for total suspended solids (TSS), chemical oxygen demand (COD), total nitrogen (TN), and total  
41 phosphorus (TP). Water and sediment transport in grass swales and filter strips were modeled by Deletic  
42 (2001). The one-dimensional model simulated runoff generation and sediment transport. The modified  
43 Green - Ampt model was used for infiltration assessment, while a kinematic wave model was used for  
44 overland-flow simulation.

45 Limitations to vegetation establishment and abundance in biofiltration swales and other vegetated storm-  
46 water facilities that treat runoff were studied through field monitoring and greenhouse experimentation  
47 (Mazer and Ewing 2001). With adequate light, vegetation and organic litter biomass was strongly  
48 inversely related to the proportion of time that the bioswales are inundated above 2.5-cm depth during the  
49 driest time of year. For most bioswales, both flow velocity and hydraulic loading were too large for  
50 sedimentation of silt and clay particles, even with dense vegetation and abundant organic litter. A lack of  
51 correlation between vegetation abundance and pollutant removal was seen.

#### 52 *Detention/Retention Ponds*

1 Design. To reduce flooding, the City of Austin, Texas initiated a watershed-based program to use regional  
2 stormwater detention facilities (Altman and Nuccitelli, 2001). The new program provided an alternative to  
3 using only on-site detention facilities. In addition, the City wanted to develop multi-objective facilities for  
4 recreation, water quality, flood-tolerant commercial uses, and/or other benefits. Dechesne et al. (2001)  
5 evaluated the long-term performance of infiltration basins by modeling their structural and environmental  
6 evolution through a LCA (Life Cycle Assessment) point of view. Sustainable infiltration basins were found  
7 to have good economic performance. Alternative configurations of detention ponds and land management  
8 plans have been generated using a genetic algorithm (GA)-based method to meet target pollutant  
9 removal levels at a relatively low cost (Harrell 2001). The GA-based method has been extended to  
10 incorporate reliability estimation into the evaluation of solutions in order to determine cost-effective pond  
11 configurations and land management plans to achieve a specified reliability level.

12 Barbosa and Hvitved-Jacobsen (2001) suggested a method for design and evaluation of the design of  
13 infiltration ponds for treating highway runoff in semiarid climates. The design was based on capture and  
14 infiltration of the most polluted runoff and accounted for the rainfall and soil hydraulic characteristics in the  
15 determination of the design volume. Seasonal variations in rainfall and evaporation were considered. Soil  
16 characteristics (hydraulic conductivity, texture, pH, and cation exchange capacity), the infiltrated volume,  
17 and the infiltrated area were used to calculate the movement of the most mobile heavy metal, Zn, in the  
18 soil below the basin. In Dubai, United Arab Emirates, high groundwater levels restrict the amount of  
19 infiltration that is permissible and influence the design of the ponds (wet versus dry) (Darnell et al.,  
20 2001b). Design criteria specific to Dubai were established to maximize the efficiency of systems that  
21 incorporate the use of detention ponds. The criteria included minimum storage pond volumes, clear times,  
22 pond geometry, water depth, the use of linings, and alternative land uses. The first pond constructed  
23 using the design criteria is now in operation collecting runoff and excess groundwater flows.

24 For more than a decade, the Queen's University/National Water Research Institute Stormwater Quality  
25 Enhancement Group studied stormwater ponds with a fully instrumented on-line system in Kingston,  
26 Ontario, Canada as a representative field installation (Anderson et al., 2001). The Group concluded that a  
27 number of identifiable factors will significantly influence the success, failure and sustainability of these  
28 ponds. These factors included initial design, operation and maintenance, performance and adaptive  
29 design. Guo and Hughes (2001) presented a risk-based approach for designing infiltration basins with  
30 design parameters of basin storage volume, drain time, and overflow risk. The risk-based approach  
31 provided an algorithm to calculate the long-term runoff capture percentage for a basin size. The  
32 diminishing return on runoff capture percentage would serve as a basis to select the proper basin storage  
33 volume at the site.

34 Cosgrove and Bergstrom (2001) reviewed the new policies and proposed regulations in New Jersey that  
35 require that no increase in stormwater pollutant loads from proposed residential and commercial  
36 developments. The authors reviewed the use of a bioretention basin as an advanced BMP that can  
37 achieve very high levels of pollutant removal. The bioretention basin would manage and treat stormwater  
38 runoff using a conditioned planting soil bed and planting materials to filter runoff captured by a collection  
39 system and transmitted to the basin. Fennessey et al. (2001) used a continuous simulation stormwater  
40 management model (with 33 years of historical precipitation) to determine how the design criteria from  
41 five different stormwater management pond ordinances changed the runoff from a 7.77 ha watershed  
42 after hypothetical development. All five ordinances required that the postdevelopment runoff rates be less  
43 than or equal to the predevelopment runoff rates for each return period. None of the five ordinances  
44 limited the increase in runoff peak rates for the 1- and 2-year return periods. This indicated that the lower  
45 frequency runoff events should always be analyzed for a basin's design.

46 Operational and maintenance practices. The objective of the study by Jacopin et al. (2001) was to  
47 develop new operational management practices for detention basins in order to limit flooding risk and to  
48 reduce pollutant discharges through optimizing the settling process. Current basin operation and "on/off"  
49 regulation studies were first carried out to quantify the freedom to act to change the control schemes.  
50 New operational rules were then elaborated and tested using a hydraulic model with their efficiency to  
51 protect against flooding and to reduce pollutant discharges being assessed.

52 Field monitoring. Konrad and Burges (2001) used a three-year rainfall record from a site in the Puget  
53 lowland, Washington in a mass-balance model to simulate outflow from single- and multiple- purpose

1 detention systems, with the results compared to time series of measured runoff from Evans Creek. The  
2 discharge from a small on-site reservoir was sensitive to both the storage capacity and maximum  
3 controlled release rate for extreme high flows (those exceeded 1% of the time) and low flows (those  
4 exceeded 80% of the time). Using wetland filtration, the North Griffin Regional Detention Pond has proven  
5 to be an effective BMP for improving water quality to receiving waters and for reducing flooding (Feldner  
6 and Greuel 2001). The 30-year-old Expo Park regional stormwater detention facility in Aurora, Colorado  
7 needed renewal (Hamilton et al., 2001). Improvements to the multi-use 60-acre park facility were made to  
8 provide water quality, improve site drainage, increase flood control detention, improve recreational  
9 usefulness and aesthetics, and upgrade the facility to meet jurisdictional State dam safety requirements.  
10 Dam safety related improvements included new outlet works, spillway improvements, and acceptance by  
11 the Engineer's Office for using irrigated turf grass as overtopping erosion protection for the emergency  
12 spillway.

13 Karouna-Renier and Sparling (2001) investigated the accumulation of Cu, Zn, and Pb by  
14 macroinvertebrates collected in Maryland stormwater treatment ponds serving commercial, highway,  
15 residential and open-space watersheds to determine if watershed land-use classification influenced metal  
16 concentrations in macroinvertebrates, sediments, and water. Composite Zn concentrations in odonates  
17 from ponds with commercial development (mean =  $113.82 \mu\text{g g}^{-1}$ ) were significantly higher than  
18 concentrations in the other land-use categories. Similarly, Cu levels in odonates from commercial ponds  
19 (mean =  $27.12 \mu\text{g g}^{-1}$ ) were significantly higher than from highway (mean =  $20.23 \mu\text{g g}^{-1}$ ) and open space  
20 (mean =  $17.79 \mu\text{g g}^{-1}$ ) ponds. However, metal concentrations in sediments and water did not differ  
21 significantly among land-uses. The levels of Cu, Zn, and Pb in invertebrates from all ponds were less than  
22 dietary concentrations considered toxic to fish. Caltrans initiated a study in two Districts (Los Angeles and  
23 San Diego, California) to examine the benefits, technical feasibility, costs and operation and maintenance  
24 requirements of retrofitting extended detention facilities into existing highway and related infrastructure  
25 (Taylor et al., 2001). Monitored constituents will include suspended solids, metals, nutrients, and organics  
26 (e.g., gasoline). Detailed records will also be kept for maintenance and operations requirements.  
27 Sampling results showed average suspended solids removal was 73%, total metals removal varied  
28 between 61% and 75%, while dissolved metals removal varied between 16% and 44%. Removal was  
29 lowest for nutrients, especially nitrate, which was about 17%. Concrete lined basin showed generally  
30 lower removal rates. Major removal of sediment is estimated to be required every 10 years.

### 31 *Wetlands*

32 It has been hypothesized that smaller lot sizes in a development need not seriously decrease quality of  
33 life but instead, may result in increased use of local open space (Syme et al., 2001). This hypothesis was  
34 investigated with matched small and larger blocks in four areas in Perth, Western Australia. Households  
35 with smaller lots had increased visitation to the wetlands. Wetlands, while improving stormwater  
36 management, also fulfilled demands caused by higher urban densities. The City of Edmonton, Alberta  
37 issued "Draft Guidelines for Constructed Stormwater Wetlands" to assist developers who use constructed  
38 wetlands for stormwater management (Lilley and Labatiuk, 2001). Edmonton has accepted constructed  
39 wetlands as an alternative means of managing stormwater, while providing benefits of aesthetics, public  
40 enjoyment and wildlife habitat. The City's draft guidelines are intended to optimize the design of  
41 constructed wetlands for water quality improvements including suspended solids removal, drainage area,  
42 soil permeability, vegetation, water depth, wetland area, wetland volume, length to width ratio, forebays,  
43 inlets and outlets, grading, floatables and oil and grease, maintenance, monitoring, signage, secondary  
44 uses, land ownership, access, fencing, wildlife and mosquitoes. The research presented by Fassman and  
45 Yu (2001) examined the relative pollutant removal capability for specific wetland vegetation. Results of  
46 water quality analysis show various trends with respect to pollutant removal and time. In general, export  
47 of TP was shown in all vegetated cells. Maximum export for vegetated cells was observed usually during  
48 the first week of a treatment, especially for *Typha* spp. and *Phragmites* spp. By the end of most  
49 treatments, TP removal was observed in all cells. COD removal was seen in all cells for all weeks of each  
50 treatment (average removal ranged from 54-78%). TSS results were inconsistent. Export of Cu was noted  
51 for *Scirpus* spp. whereas all other plants showed positive removal. TOC was removed by all plants  
52 (average removal 24 – 69%), with the exception of the *Scirpus* spp. cell.

53 Data from 35 studies on 49 stormwater treatment wetland systems were examined for obvious trends that  
54 may aid future design efforts (Carleton et al., 2001). Despite the intermittent nature of hydrologic and

1 pollutant inputs from stormwater runoff, steady-state first-order plug-flow models commonly used to  
2 analyze wastewater treatment wetlands could be adapted for use with stormwater wetlands. First-order  
3 removal rate constants for TP, NH<sub>3</sub>, and NO<sub>3</sub> for stormwater wetlands were similar to values reported in  
4 the literature for wastewater treatment wetlands. Constituent removals were demonstrated via regression  
5 analyses to be functions of the ratio of wetland area to watershed area. Goulet et al. (2001) tested a first-  
6 order removal model to predict metal retention in a young constructed wetland receiving agricultural and  
7 urban runoff. The wetland retained metals best during summer and fall. The first-order removal model  
8 predicted Fe and Mn retention in the spring and dissolved Zn retention from spring to fall in both years.  
9 However, first-order removal models failed to fit summer, fall and winter data for almost every metal under  
10 investigation (Fe, Mn, dissolved Cu, dissolved As) suggesting that HRTs (< 1-25 days) did not affect  
11 metal retention during these seasons. Therefore, the first-order removal model is inadequate to predict  
12 metal retention on a seasonal basis. Constructed wetlands design models for cold climates must consider  
13 seasonal changes that affect biological as well as hydrological variables. Wetland BMPs have focused on  
14 smaller storms, encouraging long retention times and vegetative contact for pollutant removal, design  
15 elements not typically considered for detention basins (Traver 2001). It was hypothesized that traditional  
16 storage - indication routing may not be appropriate for stormwater wetlands, especially for smaller water  
17 quality storms. To test this hypothesis, a recently constructed wetland was monitored for rainfall and flow.  
18 Sediment behavior in a stormwater wetland in Adelaide, Australia was studied using models that were  
19 based on solving hydrodynamic and transport equations, with the results compared to field-observed  
20 deposition patterns (Walker 2001). The long-term residence time distribution was useful in predicting  
21 overall sediment removal rates. Comparisons between the model and field observations indicated  
22 generally good agreement. The importance of the transient nature of the flow events was highlighted by  
23 the distribution of sediment throughout the wetland.

24 In the study by Kao and Wu (2001), a mountainous wetland located in McDowell County, North Carolina  
25 was selected to demonstrate the natural filtration and restoration system for maintaining surface water  
26 quality. Analytical results from the summer of 1997 indicated that this wetland removed a significant  
27 amount of NPS pollutants [more than 80% N removal, 91% total suspended solid removal, 59% total  
28 phosphorus removal, and 66% COD removal]. In Taiwan, research on constructed wetlands was  
29 conducted as part of a project supported by National Science Council (Yang 2001). Different waters,  
30 including contaminated river water, aquaculturing pond water, sewage, industrial wastewater, and storm  
31 water, were tested by using microcosm or macrocosm constructed wetland systems. The results of this  
32 project showed that the wetlands' effluents had a high potential for water reuse. Shutes (2001) illustrated  
33 the role of plants for treating water pollution in man-made wetlands in tropical and temperate climates.  
34 The design of the Putrajaya Lake and Wetland system in Malaysia was compared with an urban-runoff-  
35 treatment constructed wetland in a new residential development in the UK.

36 The ability of a wetland to mitigate highway-runoff pollutants, particularly metals, was investigated by  
37 Mitchell et al. (2001a). Fifty-seven rainfall-runoff events were monitored. Data collection included rainfall  
38 volume and frequency, conduit flow rate, and temperature, conductivity, pH and dissolved oxygen in the  
39 runoff. Metal concentrations were reduced via flow through the grassy median and then the wetland  
40 system. Average heavy metals removals were zinc (67%) and iron (67%), followed by lead (54%) and  
41 then nickel (45%). Metals were reduced some even during winter months and low temperatures. Wetland  
42 sediment analysis indicated a broad range of concentrations of metals. Rodgers et al. (2001) reported on  
43 the A-01 effluent outfall, which collected both normal daily process flow and stormwater runoff from a  
44 industrial park area and did not meet its South Carolina NPDES permit limits for metals, toxicity, and total  
45 residual chlorine at the outfall sampling point. Installation of a constructed wetland system including a  
46 basin to manage stormwater surges reduced the problematic constituent concentrations to below the  
47 NPDES permit limits before the sampling point. The constructed treatment wetland system proved its  
48 ability to treat industrial wastewaters containing metals with low O&M costs. With an anticipated life of  
49 over 50 years, the wetland system will be cost-effective.

50 The Greater Toronto Airports Authority (GTAA) constructed the first full-scale vertical-flow treatment  
51 wetland in Canada for reduction of ethylene glycol in stormwater (Flindall and Basran 2001). The two-cell  
52 treatment wetland was preceded by a sedimentation forebay. The wetland facility collected water  
53 discharged from terminal aprons, taxiways, and runways where aircraft receive de-icing compounds  
54 particularly glycol, are applied. The facility provided 24 to 48 h of retention to attenuate storm flows and

1 remove suspended sediment. The selected vegetation was *Phragmites australis* reeds. This emergent  
2 hydrophyte should be able to withstand the periodic flooding conditions characteristic of a stormwater  
3 management system. The reeds will provide very little habitat benefit for gulls and waterfowl and will  
4 therefore not attract birds that may cause hazards for aircraft.

#### 5 *Critical Source Area Controls – Filtration, Treatment Systems, Treatment Train Systems*

6 Bennett and Curtis (2001) tested a soil and vegetative contact treatment system (modified Howland  
7 Swale) for stormwater runoff from a fuel cell manufacturing facility. It was designed to remove copper and  
8 zinc (and thus overall effluent toxicity) to the point where the toxicity guidelines for the daphnid, *Daphnia*  
9 *pulex*, were met. The paper detailed the design and construction of the treatment system. To better  
10 understand the cost, maintenance requirements, and pollutant removal ability of sand filters, the  
11 California Department of Transportation retrofitted five Austin-style and one Delaware-style sand filters in  
12 the Los Angeles and San Diego metropolitan areas (Barrett and Borroum 2001) at their maintenance  
13 yards and park-and-ride facilities. Contributing areas averaged less than 0.7 ha. They had a relatively  
14 high cost of implementation, about \$350,000/ha treated. Pollutant removal was comparable to the  
15 performance observed in various City of Austin studies. Average suspended solids removal was 85%,  
16 total metals removal varied between 51% and 85%, while dissolved metals removal varied between 19%  
17 and 77%. Poor removal occurred for nutrients, especially nitrate, which was generally negative. No major  
18 maintenance has been required, although evidence of clogging is beginning to appear.

19 Hird et al. (2001) tested a passive treatment system, called a sorptive buoyant media clarifier (SBMC), for  
20 highway-runoff treatment. Pollutant removal efficiencies approached 95% on a mass basis total  
21 suspended solids (TSS), turbidity and particulate-bound chemical oxygen demand (COD), with the  
22 efficiencies not being affected by highly variable influents or extended periods of non-operation between  
23 storms. Prototype treatment capacity, at breakthrough, was 1000 pore-volumes treated. Liu et al. (2001a)  
24 investigated the potential of manganese oxide coated polymeric media (MOPM) for urban stormwater  
25 runoff treatment. The adsorption onto the MOPM was shown to be very sensitive to the pH, with the order  
26 of adsorption affinity on MOPM for the four divalent heavy metals studied being Pb(II) > Cu(II) > Cd(II) >  
27 Zn(II). MOPM was seen to be a viable alternative adsorption medium for heavy metal removal in upflow  
28 BMP filters, such as a SBMC. Further investigations of the surface of the MOPM (with a specific gravity of  
29 less than 1) showed that the surface area increased from 0.05 m<sup>2</sup>/g (uncoated media) to 27 m<sup>2</sup>/g for the  
30 same media after oxide coating (Liu et al., 2001b). The multiple-layer coatings enhanced filtration and  
31 adsorption of heavy metals. Investigation of materials (Liu et al., 2001c) with specific gravities greater  
32 than 1 demonstrated that cementitious materials could be coated effectively for use in stormwater  
33 treatment devices. The increase in surface area resulting from coating sand was not as significant as it  
34 was for the coating of the cementitious materials.

35 Pitt et al. (2001) reported on the results of laboratory-scale investigations into of upflow filtration as a  
36 means of increasing the run times of traditional stormwater filters (which tend to clog rapidly due to the  
37 small size of particles in urban runoff). Using laboratory-scale columns (4.8-cm inner diameter (ID)), a  
38 power equation to model downflow filtration was demonstrated. However, the coefficients for the  
39 laboratory sand filter were significantly different from those of the Lakewood, CO sand filters reported by  
40 Urbonas. Testing with a larger diameter column has been performed. Comparison of the two clogging  
41 tests indicated that a significant scale-up effect exists. Clark et al. (2001b) investigated the ability of filter  
42 media to retain previously-trapped pollutants under aerobic conditions, as would be expected if the filters  
43 were operated in an upflow mode. Permanent retention of heavy metals occurred even in an anaerobic  
44 environment. However, retention of nutrients did not occur. These results showed that aerobic conditions  
45 must be maintained in the media if nutrient removal and retention is important. The permanent retention  
46 of the heavy metals indicated that upflow filtration may be feasible where the primary stormwater  
47 pollutants are metals.

48 The use of jute and mulch as filtration media for stormwater runoff treatment was investigated by  
49 Wojtenkio et al. (2001b). The effectiveness depended on the physical characteristics of the media. Both  
50 the sand content and the particle size of the filter media affected stormwater flowrates and the media's  
51 pollutant removal capacities. Removals as high as 100 % of copper (Cu) were observed for both mulch or  
52 jute. The removal of benzo(a)pyrene (B(a)P) depended on the media-to-sand ratios and ranged from 68  
53 to 94%. Brown et al. (2001) assessed the use of kudzu (*Pueraria lobata ohwi*) as a medium for of copper,

1 cadmium, and zinc removal from low concentration waters. Kudzu was an effective adsorbent for heavy  
2 metals. Although its capacity for metals removal is less than commercial-grade ion-exchange resins,  
3 kudzu could be used at much lower cost, and may be useful in treating dilute mixed-matrix metal  
4 wastestreams, such as urban runoff, where the application of resins is not practical. Davis et al. (2001b)  
5 investigated laboratory-scale bioretention facilities to treat urban runoff. The roles of the soil, mulch, and  
6 plants for the removal of heavy metals and nutrients were evaluated. Reductions of specific metal  
7 removals ranged from 15 to 145 mg/m<sup>3</sup> per event. Moderate reductions of TKN, ammonium, and  
8 phosphorus levels were found (60 to 80%). Little nitrate was removed, with nitrate production noted in  
9 several cases. The mulch later in metal removal was found to be important.

10 Five sets of field test results of the Storm and Groundwater Enhancement System (SAGES) device were  
11 presented by Koustas and VanEgmond (2001). The SAGES device, a three-stage filtering system  
12 designed to be retrofitted to existing catchbasins/stormwater inlets, consisted of three separate filtering  
13 sacks consisting of gravel on top, sand in the middle, and granulated activated carbon on the bottom. The  
14 testing showed that the highest removal efficiencies occurred in conjunction with elevated influent  
15 suspended solids concentrations. Washout from the modified catchbasin appeared to contribute to  
16 increased effluent suspended solids concentrations that clogged the SAGES filter sacks.

#### 17 *Airport Deicer Control*

18 The paper by Switzenbaum et al. (2001) provided a theoretical assessment of the potential environmental  
19 impact of airport stormwater runoff. It also described in detail current information on alternative deicing  
20 fluid application methods and materials, collection and treatment practices.

#### 21 *Combined Sewer Overflow/Sanitary Sewer Overflow Control*

22 Innovative CSO Controls – Source Controls. The methodologies used by several sewerage agencies to  
23 control combined sewer overflows (CSOs) and sanitary sewer overflows (SSOs) were  
24 presented/reviewed during 2001. Jewell (2001) outlined the methodology used by the Boston Water and  
25 Sewer Commission to identify and correct illicit connections to the storm drain system, as detailed in the  
26 Massachusetts Water Resources Authority's Combined Sewer Overflow Facilities Plan. The Plan  
27 proposed improvements to the system infrastructure to reduce the frequency and duration of CSOs. The  
28 CSO control plan proposed by the City of Springfield, Massachusetts to meet EPA and Massachusetts  
29 DEP requirements was reviewed by Heath and Donahue (2001). The cost for the various alternatives  
30 ranged from \$144 million (reduce discharges to 12 per year) to \$525 million (complete sewer separation  
31 to eliminate CSOs). Economic analyses were coupled with a regional water quality model to evaluate the  
32 benefits of various proposed alternatives for CSO control. Lai et al. (2001) reviewed the SSO Control Plan  
33 developed for Henrico County, Virginia. The paper discussed the following components: (1) dry-weather  
34 wastewater flow projections using GIS, water consumption records, and flow monitoring; (2) rainfall-  
35 dependent infiltration/inflow predictions; (3) capacity analysis (present and future conditions) using the  
36 SWMM EXTRAN model; (4) hydraulic and process capacity analysis of the wastewater treatment facility,  
37 including expansion requirements; and (5) implementation of a capital improvement program for  
38 additional facilities (sewers, storage tanks, pump stations, force mains, and wastewater treatment plant  
39 expansion or process retrofitting). O'Rourke et al. (2001) presented the Northeast Ohio Regional Sewer  
40 District's long-term control plan (LTCP) for bringing the District into compliance with EPA's CSO control  
41 policy. This approach included traditional and non-traditional cost issues, including non-cost issues that  
42 were deemed important by the public. The paper presented by Stupar and Dierksheide (2001) reviewed  
43 the new integrated maintenance, inventory and purchasing system implemented by the Ocean County  
44 Utilities Authority. The goal of integrating the system was to not only ensure that high quality WWTP  
45 effluent is discharged but also to reduce the probability of sanitary sewer overflows. Scott et al. (2001)  
46 reported on the Emergency Response Plan implemented by the City of Baltimore, Maryland as a  
47 response to the EPA's guidelines for developing a CMOM program. In this Plan, critical facilities (pumping  
48 stations, pipelines, valves, and other components with a high risk of failure) were identified and their  
49 impact on public health and the environment was ranked. This Plan is guiding the implementation of  
50 recommended mitigation measures.

51 The use of analyzing utility data relative to sanitary sewer overflows was presented by Nelson et al.  
52 (2001). The purpose was to establish protocols for identifying SSOs through data already collected by  
53 utilities. Turner et al. (2001b) reported on Columbus, Georgia's development of plans for managing CSOs

1 and SSOs. The strategy was based on the “further-reasonable-progress” approach to attaining water  
2 quality standards. The keys to success have been the demonstration of sustainable watershed solutions  
3 and the results from continued receiving water monitoring. The EPA BASINS model, with field-data  
4 calibration, was used to digitally represent the watershed and evaluate potential improvements. According  
5 to McConico et al. (2001a), in 2000, the City of Richmond, Virginia conducted a bacteriological  
6 monitoring. The purpose of this data collection was to monitor the effectiveness of the City’s long-term  
7 CSO control program through comparing the new data to data collected in the 1980s and 1990s. The  
8 data was also used in the combined sewer system and James River tidal models that predicted for the  
9 effectiveness of long-range CSO planning. Carr (2001) reviewed the concept of micromanagement of  
10 stormwater flows to reduce SSOs and CSOs. Micromanagement of stormwater advocates temporarily  
11 storing stormwater in many and varied locations on the surface (off-street and on-street) and, as needed,  
12 below the land surface, near to the source. The paper reviewed the application of stormwater  
13 micromanagement in Skokie, Illinois and the lessons learned from the implementation.

14 Sewerage Repairs and I/I. As reported by Lai and Field (2001), the USEPA has conducted a series of  
15 research, development and demonstration projects on the characterization, cause, consequence, and  
16 control of infiltration/inflow (I/I) in both sanitary and combined sewers. The research addressed (1) state-  
17 of-the-art problem assessment; (2) pressure sewer systems; (3) polymers to increase sewer carrying  
18 capacity; (4) sealing methods and materials for sewer rehabilitation; (5) demonstration and evaluation of  
19 Insituform; (6) trenchless sewer installation by “plowing in;” (7) house lateral rehabilitation; and (8)  
20 impregnated concrete pipe to increase corrosion resistance and strength. This review was designed to  
21 assist communities that will be implementing the soon-to-be-issued SSO Rule. Moisio and Barton (2001)  
22 reviewed the design standards for sewers used by the Metropolitan Sewer District of Greater Cincinnati  
23 (MSDGC). The MSDGC maintenance program was designed to avoid SSOs due to poor maintenance.  
24 What had been documented was that replacement pipes were designed to the standard of new sewers  
25 and therefore I/I for the older sewer system was not accounted for in the calculations. The new revisions  
26 to the design standards will incorporate I/I contributions for older areas, thus allowing the installation of  
27 replacement pipes that can carry the flows from older areas. O’Sullivan et al. (2001) reviewed the  
28 implementation of a private sanitary sewer lateral replacement program (SSLRP) in Mobile, Alabama. The  
29 SSLRP performed in two distinct processes that used separate contractors – the testing and identification  
30 of defective private sanitary sewer laterals (PSSLs) and the replacement of defective PSSLs essentially  
31 at the cost of the property owner. A detailed program methodology, consistent penalty system, and close  
32 coordination between Water and Sewer System, the engineer, and the contractors were the keys to the  
33 program’s success.

34 Hilderhoff and Wendle (2001) recommended a mini-basin or mini-watershed approach to sewer  
35 rehabilitation, as was used by Susquehanna and Lower Paxton Townships. Total rehabilitation by mini-  
36 basin means that all sewer system components including mainline, manholes, service laterals and  
37 building sewers located in one mini-basin were repaired to meet the same acceptance testing standards  
38 as new sewers. This work was completed at the expense of each Authority and not the property owner.  
39 Blakley and Summers (2001) reviewed the approach taken by the McCandless (Pennsylvania) Township  
40 Sanitary Authority for I/I control and recommended this synergistic approach to other communities. The  
41 approach combined strengths of five different strategies (dye testing, line replacement, pipe relining, line  
42 grouting and manhole rehabilitation), creating a synergistic effect, that balanced cost with gain. Field and  
43 O’Connor (2001) recommended communities develop a strategy for SSO pollution abatement because  
44 extensive sanitary sewer rehabilitation without planning is (1) relatively costly, (2) time-consuming, and (3)  
45 extremely disruptive to traffic, property owners, etc. I/I control studies have demonstrated that just  
46 correcting I/I in street sewers will not necessarily correct the problem because building connections  
47 contribute as much as 60% of the infiltration load. Building connection rehabilitation may be unfeasible  
48 economically. Inflow elimination or reduction, cost-effective sewer rehabilitation, and collection system  
49 inspection with associated clean out and repair must be performed in all cases, and must be part of an  
50 integrated economic and feasibility analysis.

51 Lukas et al. (2001) reviewed the WERF project to identify and develop Predictive Methodologies for  
52 Determining Peak Flows after Sanitary Sewer Rehabilitation Projects. The first result of the municipal  
53 surveys was the lack of documentation on rehabilitation projects and particularly, their effectiveness. The  
54 paper by Watts and Forbes (2001) analyzed the procedures used to rate I/I defects located during the

1 recent Sanitary Sewer Evaluation Studies (SSES) conducted in Carolina and Luquillo, Puerto Rico. The  
2 methods used to prioritize appropriate rehabilitation methods were presented. Kurz et al. (2001) reviewed  
3 the City of Chattanooga's permanent network of flow meters for monitoring sewer flows as part of their  
4 billing. In addition, the flow meter data assisted the city in its CMOM program by locating capacity  
5 problems. Nashville's flow metering system identified over 10 million m<sup>3</sup> of I/I removal after rehabilitation.  
6 Jackson, Tennessee's flow meters demonstrated that I/I rehabilitation solved what appeared to be a  
7 capacity problem in a major trunk line. Hollenbeck and Rieger (2001) outlined the Rock River Water  
8 Reclamation District program that was designed to mitigate basement backups in the District. This  
9 program identified and rehabilitated both public and private sources of infiltration in the study area. Post  
10 rehabilitation flow monitoring confirmed that 10-year storm protection from overflows and basement  
11 backups was achieved.

12 Public Education. White (2001) reviewed the joint agency program of King County and the City of Seattle  
13 (in conjunction with the Seattle-King County Health Department) for public notification of combined sewer  
14 overflows. The program included posting warning signs at over 100 CSO locations, developing a website,  
15 information line, brochure, fact sheet and other materials. According to the review of Rozelman and  
16 Loncar (2001), the 1995 New York State Discharge Notification Act (DNA) (aka "Fisherman's Right to  
17 Know Act") caused the New York City Department of Environmental Protection to enhance their public  
18 education program regarding CSOs and encouraged the public to provide the City with early notification  
19 of sewer problems such as dry-weather discharges.

20 Tunnels and Interceptors. Three major tunnel construction projects were presented during WEFTEC  
21 2001. Jones and Robison (2001) reviewed the history, the design, and the construction of the  
22 Chattahoochee Tunnel in Metropolitan Atlanta. This tunnel was designed to accomplish two purposes:  
23 relieve the existing Chattahoochee Interceptor and provide flow equalization for the existing water  
24 reclamation facility. Wood et al. (2001) presented an overview of the decisions that went into the design  
25 of the interconnected Westside and Eastside CSO tunnels in the City of Portland. The analyses included  
26 comprehensive hydrologic and hydraulic modeling of the combined sewer drainage system and the tunnel  
27 system, RTC simulation of gates, weirs, and pumping facilities, and planning for instrumentation,  
28 monitoring, operation, control, and maintenance. Roll and Lannon (2001) reviewed the repair of the Falls  
29 Street Tunnel in Niagara Falls, New York. The repair was designed to reduce the 6-MGD of groundwater  
30 infiltration into the tunnel. The contractor payment for the project was based on the infiltration reduction,  
31 which was estimated to be approximately 4 MGD.

32 Ab Razak and Christensen (2001) investigated the benefits of the Milwaukee deep tunnel for temporary  
33 storage of storm water and sewage, on the water quality of the Milwaukee, Menomonee, and Kinnickinnic  
34 Rivers. Statistical analysis of BOD, phosphorus, suspended solids, fecal coliforms, zinc, and chloride  
35 indicated that the Menomonee River benefited the most from the deep tunnel since 1994 when the tunnel  
36 came on line. Fecal coliforms inside the CSO area, and to a certain extent BOD and zinc levels, exhibited  
37 the most significant decline.

38 Litter, Floatables and Settled Solids. The paper by Fan et al. (2001a) reviewed the causes of sewer  
39 deterioration and the control methods that can be used to prevent or postpone this deterioration. The  
40 remedy reviewed in the paper was inline and combined sewer overflow tank flushing systems to remove  
41 sediments and thus minimize hydrogen sulfide production in the sewer system. The two technologies  
42 reviewed were the tipping flusher and the flushing gate, both of whom have been demonstrated to  
43 perform well in facilities in Germany, Canada and the United States.

44 Real-Time Control (RTC). O'Connor and Field (2001) reviewed the USEPA Capstone Report on control  
45 system optimization, including a management strategy that will maximize the use of the existing system.  
46 This will postpone construction of new facilities and will size storage volumes in concert with the treatment  
47 rate to obtain the lowest-cost storage-treatment system. The components, hardware, and strategies to  
48 create such a system were described. An RTC methodology was used to measure the discharge problem  
49 from an urban area (Yamada et al., 2001b), and was used in conjunction with a stormwater tank to retain  
50 flows. The results showed that COD in the stormwater runoff was reduced to approximately 0.45 mg/L.

51 The sewer system evaluation by the Milwaukee Metropolitan Sewer District (MMSD) proposed that flow  
52 regulation in the system be controlled not by specific flow rate from the tributaries but instead by  
53 maintaining a constant hydraulic grade line in the system, i.e., the flow allowed in the system will be

1 determined by the available capacity of the downstream pipes (Bate 2001). The paper by Schultz et al.  
2 (2001a) explained how controlling the interceptor system, rather than just relieving it, enabled the  
3 Milwaukee MSD to achieve far more than merely reducing bypass and overflow frequency. The collection  
4 systems controls allow the MMSD to meet tight overflow restrictions in a manner flexible to variable  
5 precipitation patterns, growth patterns, and lake and groundwater levels. In addition to more efficient use  
6 of intercepting sewer capacity, the regulation strategy also provided a significant cost savings when  
7 compared to conventional flowrate regulation. The Louisville and Jefferson County Metropolitan Sewer  
8 District (MSD) is considering Real Time Control as a possible alternative in the Combined Sewer  
9 Overflow (CSO) Long Term Control Plan (LTCP) (Charron et al., 2001). Two successive studies have  
10 produced very promising results – the first implying an overall reduction of 64% in annual overflow, and  
11 the second currently showing similar results to the first.

12 Chowdhury et al. (2001) reviewed the RTC control model developed for the District of Columbia. The  
13 model was applied to evaluate existing and potential improvements to the system, and also used to  
14 evaluate the effectiveness of previously-installed controls. The model results indicated that significant  
15 reductions in overflows had been achieved through the controls installed in the past. Real-time control  
16 (RTC) operations strategies for the Quebec Urban Community (QUC) were discussed by Colas et al.  
17 (2001). Simulation of each RTC strategy using 32 real back-to-back rainfalls showed that the existing  
18 system performs better under RTC than with conventional control. Efficient in-system storage and  
19 treatment capacities and the elimination of flood risks were the main benefits of this type of real-time  
20 management. The results of the first year of operation of the optimized real-time control system for the  
21 QUC were presented by Lavallee et al. (2001). These results confirmed the model results for the benefits  
22 of the QUC RTC. RTC use in Philadelphia was reviewed by Marengo et al. (2001). The implemented RTC  
23 system provides the capability to prioritize capture of combined flow in the collection system, to minimize  
24 overflows to sensitive receiving streams, and to optimize the use of available interceptor sewer-system  
25 storage during wet weather. SWMM model simulations were performed to quantify the benefits of RTC  
26 implementation in PWD's SWDD.

27 CSO Storage Tanks and Structures. Carr et al. (2001) reviewed the use of street-storage systems to  
28 prevent combined sewer overflows and mitigate basement flooding, based on the idea of accepting the  
29 complete volume of water but reducing the inflow rate of the stormwater to the sewer system. System  
30 components included street berms, flow regulators and surface and sub-surface storage facilities.  
31 Charlotte-Mecklenberg Utilities also found that they were able to service new development without  
32 creating overflows by storing peak flows upstream (Crowley and Howard 2001), thus delaying a system-  
33 wide expansion. Carter et al. (2001) reported on the evaluation of inflatable dams for use in Philadelphia  
34 as part of its Long Term Combined Sewer Overflow Control Plan (LTCP). These dams were operated  
35 under real-time controls to storage and treat flows upstream, thus reducing the frequency and volume of  
36 CSOs. Based on model results (using EXTRAN) with three inflatable dams operated by RTC, a 70% (50-  
37 million gallon) reduction in average annual CSO volumes to the Schuylkill River would be expected.

38 Murphy and Ring (2001) reviewed the use of pre-cast concrete box sections to create CSO storage  
39 facilities. The savings from using pre-cast versus cast-in-place boxes was estimated at \$3.1 million dollars  
40 for the 1.2-million gallon Kenduskeag East CSO Storage Facility. The Pollution Control Plan (PCP) for  
41 Hamilton-Wentworth recommended the construction of 10 – 12 underground storage tanks to intercept  
42 CSO waters for subsequent conveyance to the treatment plant (Stirrup 2001). The sizing of the tanks was  
43 performed using SWMM. The premise was that continuous modeling would be the only reliable way to  
44 ensure that the tank performs to those control criteria under real operating conditions for long periods of  
45 time. Freedmand et al. (2001) reported on the use of unused aeration basins for receiving and equalizing  
46 industrial and CSO flows to the treatment plants in Rockland, Maine.

47 CSO Treatment. Wojtenko et al. (2001a) reviewed the chapter of the EPA Capstone Report on control  
48 and treatment of CSOs, SSOs, and stormwater runoff. The paper discussed inline and offline storage  
49 systems as well as in-receiving water storage systems. Three major types of treatment systems and their  
50 associated costs were also discussed: (1) physical (screening and sedimentation), including comparing  
51 high-rate vs. conventional processes; (2) physical/chemical (filtration and high-rate sedimentation); and  
52 (3) biological (trickling filters, activated sludge, aerated lagoons, and rotating biological contactors).

1 Project Review – General. Gaffoglio et al. (2001) reviewed the City of New York’s \$1.8 billion Citywide  
2 CSO Program that addressed CSO discharges from the more than 450 CSO locations within the City.  
3 Four open water projects (East River, Jamaica Bay, Inner Harbor and Outer Harbor), and four tributary  
4 projects (Flushing Bay, Paerdegat Basin, Newtown Creek, and Jamaica Tributaries) were included in the  
5 program. The paper described the CSO facilities NYC is building and planning under this major capital  
6 planning and construction effort, and it discussed the challenges faced by the City in implementing the  
7 program. Mynhier et al. (2001) summarized the City of Atlanta’s recommendations for their Long-Term  
8 Control Plan for CSOs. The City proposed three options ranging from separating about 80 percent of the  
9 CSO service area (\$1.25 billion) to solely adding large consolidated storage and treatment facilities (\$710  
10 million). The preferred option was between the two extremes, with some separation and consolidated  
11 storage and treatment facilities capable of treating all storm water runoff from the CSO area (\$950  
12 million). Turner et al. (2001a) reviewed the CSO technology-testing program implemented in Columbus,  
13 Georgia. Results included protocols for characterization, model calibration and watershed yield  
14 quantification. Overall program findings included load generation rates and yields for aquatic biology  
15 indices. CSO treatment demonstrations indicate high-rate compressed media filtration as a cost-effective  
16 technology that may also be applied to stormwater as an impervious area flush-control. Schenk et al.  
17 (2001b) presented an overview of the EPA’s Environmental Technology Verification (ETV) Program, with  
18 a focus on two areas of technology: flow meters and stormwater source-control devices. The Wet  
19 Weather Flow Technologies Pilot (WWF Pilot) program balances the desire for comprehensive testing  
20 (full characterization of equipment performance under a variety of water quality and facility conditions)  
21 against the need to keep testing costs reasonable.

22 Disinfection. Atasi et al. (2001c) presented the work performed by the City of Detroit Water and Sewerage  
23 Department (DWSD) that evaluated chemical disinfectants for the Baby Creek CSO water. EPA’s ETV  
24 Program developed generic protocols for the testing of mixing needs in high-rate disinfection, such as that  
25 needed for CSO treatment (Moffa et al., 2001). The ETV program is also investigating alternative  
26 disinfectants. The New York City pilot study addressed, and an ongoing WERF project is addressing,  
27 alternative technologies. As part of the ETV Program, the WWF Pilot investigated the potential of high-  
28 rate disinfection technologies for wet-weather flow applications (Schenk et al., 2001a). The benefits of  
29 using simulated flows were described, as were the potential limitations associated with applying the data  
30 to actual wet weather collection systems. Wojtenko et al. (2001c) presented the results of several major  
31 pilot-scale studies to evaluate the effectiveness of UV light to disinfect CSO flows. UV light irradiation,  
32 when correctly applied, was found to be an effective alternative to chlorination for CSO disinfection.  
33 However, the success of disinfecting with UV light seemed to be strongly dependent on water quality.  
34 Wojtenko et al. (2001d) reviewed the chapter of the EPA Capstone Report on disinfection. Locating  
35 effective alternate disinfectants has proved to be very difficult.

36 High-rate clarification (including ballasted flocculation). Jolis and Ahmad (2001) reviewed the use of high-  
37 rate clarification at the wet-weather treatment plants in San Francisco. The process was found to be  
38 highly effective for CSO pollutant removal. Suspended solids removal in excess of 80% of influent  
39 concentrations were achieved consistently. COD and BOD<sub>5</sub> removal exceeded 60%. Keller et al. (2001)  
40 reviewed the design of a ballasted flocculation system for the Lawrence (Kansas) WWTP. The ballasted  
41 flocculation system was used to treat excess flows from wet-weather events. The paper reviewed the jar  
42 test results, the performance of liquid versus dry polymers, and other design issues. The City of Rocky  
43 River, Ohio reduced their SSOs through expansion of the WWTP’s primary treatment capacity (Leffler  
44 and Harrington 2001). The primary-treated effluent in excess of what the secondary treatment could  
45 handle was then blended with fully-treated effluent waters prior to discharge to the receiving water. This  
46 blending allowed the overall plant effluent to meet regulatory standards with the concurrent elimination of  
47 headworks bypassing. Ballasted flocculation to improve WWTP performance during wet-weather flow  
48 periods was reviewed by Scruggs and Wallis-Lage (2001). Based on pilot and bench scale testing  
49 performed at three facilities, the ballasted flocculation technology was found to be compatible with client  
50 needs. In all cases, the ballasted flocculation system removed 75 to 95% of influent storm flow TSS at  
51 hydraulic loading rates varying from 60 to 80 gpm/sf when ferric chloride was used as the coagulant. The  
52 use of ballasted flocculation was also cost-effective at the three plants.

53 Filtration. Three western New York State treatment plants have retrofitted their existing sand filters to a  
54 coarse monomedia (Bentivogli and Smith 2001). The new media filters use less peak backwash water,

1 have increased filter run times and have maintained the same effluent quality as the older sand filters with  
2 the smaller diameter media. Testing is ongoing for CSO treatment. The Niagara County Sewer District  
3 No. 1 Water Pollution Control Center installed mono media filtration to increase capacity in order to treat  
4 additional flows during wet-weather events. The result is that the Sewer District has met their goal of  
5 increasing filtrating capacity and filter run times, while eliminating filter bypassing.

6 *Flow control in WWTP.* In Flanders, Belgium, the potential to treat additional flow in the biological system  
7 of a WWTP was investigated by Carrette et al. (2001). The concept was that higher hydraulic loadings  
8 could be treated within the biological treatment area if additional secondary clarifier volume was supplied.  
9 This operation scenario was successful and the overall pollutant discharge was significantly reduced.  
10 Chen and Beck (2001) also reported on using practical controllers to maximize the flow through the  
11 biological treatment system of a WWTP, with the result being the minimization of bypass flows. The  
12 results of the tests illustrated that the tested controllers could regulate the flows through activated sludge  
13 process sufficiently to maximize the treatment plant performance. Niemann and Orth (2001) modeled  
14 three modifications to traditional WWTPs in order to maintain a high efficiency of treatment during wet-  
15 weather flows. The three measures were increasing the nitrification volume, bypassing the primary  
16 sedimentation, and adding flocculants before secondary sedimentation. All three measures were  
17 effective.

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